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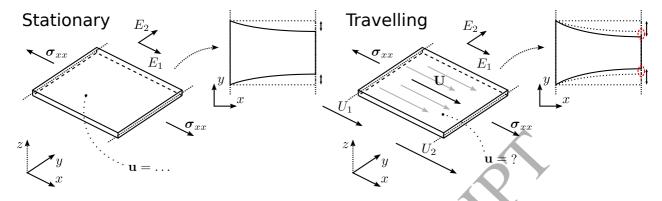
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#### **Graphical Abstract**



### Highlights

- In-plane motion of an axially travelling orthotropic sheet was modelled
- Velocity difference between supports generates strain due to mass conservation
- In a travelling viscoelastic material the axial strain varies along the length
- Small deformations of free edges due to the Poisson effect affect the velocity
- Inertial effects lead to additional cross-directional contraction

# The Origin of In-plane Stresses in Axially Moving Orthotropic Continua

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#### **Abstract**

In this paper, we address the problem of the origin of in-plane stresses in continuous, two-dimensional high-speed webs. In the case of thin, slender webs, a typical modeling approach is the application of a stationary in-plane model, without considering the effects of the in-plane velocity field. However, for high-speed webs this approach is insufficient, because it neglects the coupling between the total material velocity and the deformation experienced by the material. By using a mixed Lagrange–Euler approach in model derivation, the solid continuum problem can be transformed into a solid continuum flow problem. Mass conservation in the flow problem, and the behaviour of free edges in the two-dimensional case, are both seen to influence the velocity field. We concentrate on solutions of a steady-state type, and study briefly the coupled nature of material viscoelasticity and transport velocity in one dimension. Analytical solutions of the one-dimensional equation are presented with both elastic and viscoelastic material models. The two-dimensional elastic problem is solved numerically using a nonlinear finite element procedure. An important new fundamental feature of the model is the coupling of the driving velocity field to the deformation of the material, while accounting for small deformations of the free edges. The results indicate that inertial effects produce an additional contribution to elastic contraction in unsupported, free webs.

*Keywords:* axially moving, orthotropic, viscoelastic, elastic, one-dimensional, two-dimensional, free edges

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#### 1. Introduction

In the handling of continuous, high-speed webs the origin of in-plane stresses creates a scientific problem, which is not yet completely understood. Especially, the type of the web material has a significant effect on both qualitative and quantitative characteristics of the in-plane stresses. In moving continuous web systems, the tension of the web can usually be controlled in the direction of the transport velocity, the tension being generated by a velocity difference between the starting and ending lines of a free span. At high transport velocities, both web stress and web stability are of concern, not only in this longitudinal direction, but also in the direction perpendicular to the main transport velocity in the plane of the web.

Axially moving materials have many applications in industry, e.g. in paper production, and their mechanics have been studied widely. In the processing of different kinds of thin, laterally moving solid webs, challenges are met, such as the efficiency of production and effects caused by the high processing speed. The first studies of the vibrations of travelling elastic materials date back to the end of the 19th century (Skutch, 1897 [63]) and to the middle of the 20th century (Sack, 1954 [57]; Archibald and Emslie, 1958 [1]). A string model for the moving material was used in all of these studies. Later on, in the 1960s and 1970s, many researchers continued studies on moving strings and beams, concentrating mainly on various aspects of free and forced transverse vibrations (e.g. Miranker [46], Swope and Ames [66], Mote et. al. [48, 49, 50] and Simpson [61]).

The stability of small transverse vibrations of travelling two-dimensional rectangular membranes and plates have been studied by Ulsoy and Mote [71], and Lin [40]. When the web is advancing through a process without external support, the inertial forces depending on the web speed are coupled with web tension. Also the transverse behaviour of the web and the response of the fluid (air) surrounding the web are coupled (see e.g. [9, 54]). Studies modelling the moving web coupled with the surrounding air have been made by Pramila et al. [51, 55, 33]. In their studies, it was found that the surrounding air significantly reduces the eigenfrequencies and critical velocities of the web, when compared to the vacuum case. Chang and Moretti [9] studied membranes using potential flow theory, and Banichuk et al. [5, 6] used the flat panel model coupled with potential flow. This research was extended by Jeronen [31], where the eigenfrequency spectra were investigated for this model and for the moving string with damping. In Watanabe et al. [72], two different methods of analysis were developed for the phenomenon of paper flutter. One of these was a flutter simulation using a Navier-Stokes code, and the other method was based on a potential flow analysis of an oscillating thin airfoil.

Lin and Mote studied an axially moving membrane in a 2D formulation, predicting the equilibria of the displacement and the stress distribution under transverse loading [41]. Later, they continued studying the wrinkling of axially moving rectangular webs with a small flexural stiffness [42]. They predicted the critical value of the non-linear component of the edge loading after which the web wrinkles, and the corresponding wrinkled shape of the web. It is also known that the lack of web tension will result in a loss of stability of the moving web, which from the application

viewpoint, disturbs the required smooth advancing of the web (see e.g. [4, 3]). On the other hand, high tension may cause web breaks, which deteriorates production efficiency (see e.g. [2, 56, 58, 62]).

Paper has often been modelled as an orthotropic elastic solid. Elastic constants have been measured for some paper-like materials by Mann, Baum and Habeger [43]; and Baum, Brennan and Habeger [7]. Recently, for anisotropic solids, Erkkilä et al. [18] have studied competent parameters based on modeled stress-strain curves for further construction of a material model. Out-of-plane Poisson ratios, specifically, have been recently studied by Stenberg and Fellers [65], who reported that paper is an auxetic material: stretching in the machine direction will cause the paper web to thicken in the out-of-plane direction. The relevant Poisson ratio,  $\nu_{13}$ , is negative, and  $|\nu_{13}|$  may be as large as 3.0. Incompressible and slightly compressible orthotropic and transversely isotropic materials have been investigated by Itskov and Aksel [30], who discovered nontrivial conditions that the elastic constants must satisfy in order to obtain incompressible or slightly compressible behaviour.

Considering wet paper material, the viscoelastic properties play an important role in the behaviour of the web, and thus, need to be included in the model. The first study on transverse vibration of travelling viscoelastic material was carried out by Fung et. al. using a string model [20]. Extending their work, they studied the viscous damping effect in their later research [21]. Viscoelastic strings and beams have recently been studied extensively, see e.g. [45, 74]. Oh et al. studied critical speeds, eigenvalues and natural modes of the transverse displacement of axially moving viscoelastic beams using the spectral element method [39, 52]. Chen and Zhao [14] presented a modified finite difference method to simplify a non-linear model of an axially moving string. They studied numerically the free transverse vibrations of both elastic and viscoelastic strings. Chen and Yang studied free vibrations of viscoelastic beams travelling between simple supports with torsion strings [13]. They studied the viscoelastic effect by perturbing the similar elastic problem and using the method of multiple scales. Very recently, Yang et al. studied vibrations, bifurcation, and chaos of axially moving viscoelastic plates using finite differences and a non-linear model for transverse displacements [73].

Marynowski and Kapitaniak studied differences between the Kelvin–Voigt and Burgers models in the modelling of the internal damping of axially moving viscoelastic beams. They found out that both models gave accurate results with small damping coefficients, but with a large damping coefficient, the Burgers model was more accurate [44]. In 2007, they compared the models with the Zener model studying the dynamic behaviour of an axially moving viscoelastic beam [45]. They found out that the Burgers and Zener models gave similar results for the critical transport speed whereas, the Kelvin–Voigt model gave a greater critical speed compared to the other two models.

The origin and structure of the in-plane stress and strain distribution in a moving solid web seems to be an exceptionally unknown area. The models used with web materials are often based on assumptions of isotropic or orthotropic material properties (see e.g. [7, 68]). The material is considered as viscoelastic or viscoplastic,

but in the models, there is usually no coupling between the in-plane strain and the web velocity effects (see e.g. [24, 53, 70]).

Time-dependent, in-plane vibrations of a moving continuous membrane were studied by Shin et al. [59]. In their work, in-plane vibration modes of an isotropic web were studied between the traction lines. Also Guan et. al. have studied viscoelastic web behaviour in both steady state and unsteady state cases [22, 23]. In the recent years, the topic of supercritically moving materials has received attention; for example, see Ding et al. [16], where steady-state periodic responses regarding time-harmonic forced vibrations of supercritically moving viscoelastic beams were studied using multiscale analysis and finite differences. Note, however, that in the present study, we will concentrate on the subcritical regime, which is more relevant for paper production.

Traditionally, the partial time derivative has been used instead of the material derivative in the viscoelastic constitutive relations, but Mockensturm and Guo suggested that the material derivative should be used [47]. They studied non-linear vibrations and the dynamic response of axially moving viscoelastic strings. Kurki and Lehtinen also independently suggested that the material derivative should be used in the constitutive relations, in their study concerning the in-plane displacement field of a travelling viscoelastic plate [37, 35].

In a study by Chen et al., the material derivative was used in the viscoelastic constitutive relations [10]. They studied parametric vibration of axially accelerating viscoelastic strings. Chen and Ding studied the stability of axially accelerating viscoelastic beams using the method of multiple scales, and the material derivative was used in the viscoelastic constitutive relations [15]. Chen and Wang studied the stability of axially accelerating viscoelastic beams using asymptotic perturbation analysis and the material derivative in the viscoelastic relations [12]. The material derivative was also used in a recent paper by Chen and Ding, where the dynamic vibration response of axially moving viscoelastic beams was studied [11]. A non-linear model was used, taking into account the coupling of the transverse displacement with the longitudinal (in-plane) displacement. However, the transverse behaviour of the beam was their main focus.

In this paper, we propose to modify the classical two-dimensional model of a moving viscoelastic web by accounting for the coupling between the velocity field and the in-plane strain. A two-dimensional, thin open loop (non-conservative system) made of an orthotropic membrane is stretched using a relative speed difference between the traction lines. The orthotropic viscoelastic material assumption is applied, using a viscoelastic model of the Kelvin–Voigt type. The axial motion is accounted for by using a moving reference state that translates axially at constant velocity (see e.g. [67, 34]). This method handles the behaviour of a solid moving web using a control volume approach similar to the treatment of fluid flows. Preliminary one-dimensional studies have been reported in the paper [36]. In the present paper, the steady state of the two-dimensional moving continuum, in the pure elastic case, is solved using the nonlinear finite element method. An important new fundamental feature of the considered model is the coupling of the driving velocity field to

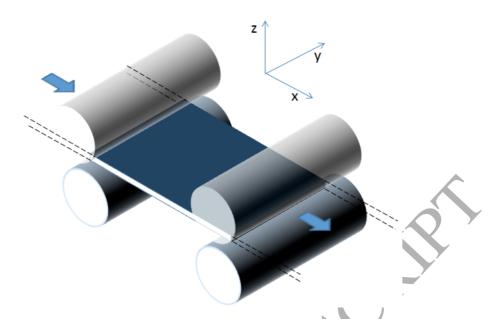


Figure 1: Schematic representation of the setup for modelling a moving viscoelastic web, stressed at the traction lines represented by the rollers. The arrows depict axial motion.

the deformation of the material, while accounting for small deformations of the free edges.

#### 2. Strain generated by velocity difference of subsequent rollers

A continuous, moving web creates a flow continuum which is possible to consider as a solid medium experiencing flow. Due to its solid nature, the web continuum is always under a stress state, which is caused by a strain state, which further can be expressed in terms of the velocity difference between subsequent supporting rollers, by the means of a mass conservation argument.

Consider an orthotropic material having initially (i.e. in the undeformed state) constant density  $\rho_0$ , undergoing steady-state longitudinal transport at velocity  $\mathbf{U} = (U_x, 0, 0)$ , depicted in Figure 1. Let us assume that the material axes 1, 2 and 3 are aligned with the global coordinate axes x, y and z, respectively. The continuity equation, as expressed in the Eulerian frame, is

$$\frac{\partial \rho}{\partial t} + \nabla \cdot (\rho \mathbf{U}) = 0 , \qquad (1)$$

where  $\rho$  is density of the material, and  $\partial/\partial t$  is the partial time derivative in the Eulerian frame. In a steady state, the equation reduces to

$$\nabla \cdot (\rho \mathbf{U}) = 0 . \tag{2}$$

Mass conservation requires that the flow rates at the incoming and outgoing flow control areas match; this requirement is readily obtained from (2). Let us consider a

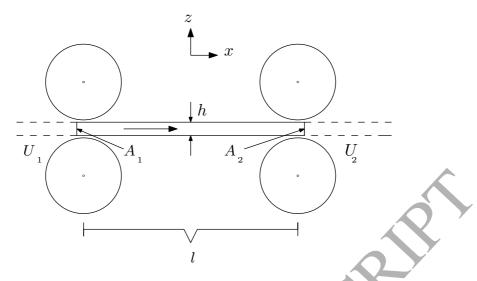


Figure 2: Solid web continuum flowing between the incoming and outgoing flow control areas (two-dimensional surfaces)  $A_1$  and  $A_2$ , at longitudinal speeds  $U_1$  and  $U_2$  at the beginning and ending traction lines (respectively).

stationary control volume

$$\Omega = \{ (x, y, z) : 0 < x < \ell, \ 0 < y < b, \ 0 < z < h \} ,$$

where  $\ell$  is the length of the span between the rollers, b is the width of the span, and b is the thickness of the sheet of material. Integrating (2) over the control volume  $\Omega$ , applying the divergence theorem, and noting that  $\rho$  is a scalar, we have

$$\int_{\Omega} \nabla \cdot (\rho \mathbf{U}) \, d\Omega = \int_{\partial \Omega} \rho \left( \mathbf{n} \cdot \mathbf{U} \right) \, d\Gamma \,, \tag{3}$$

where  $\partial\Omega$  is the closed surface delimiting the control volume  $\Omega$ , and n represents outer unit normal vector. The differential  $d\Gamma$  refers to integration over a surface.

As is shown in Figure 2, flows in and out of the control volume occur only at the surfaces  $A_1$  and  $A_2$ . Let us assume that  $\rho$  and U are constant across these inand outflow surfaces, but that their values may change between these surfaces. In practice, there may exist small variations in the velocity at the outlet due to material straining, but for small strains and high velocity, constant velocity along these surfaces is a reasonable approximation.

Under these assumptions, the mass balance in (3) becomes

$$-\rho_1 A_1 U_1 + \rho_2 A_2 U_2 = 0 . (4)$$

The subscripts 1 and 2 for  $\rho$  and U refer to the (constant) values on the surfaces  $A_1$  and  $A_2$ , respectively. Note the form of the velocity field,  $\mathbf{U} = (U_x, 0, 0)$ . Finally, it is convenient to rewrite (4) as

$$U_1 \frac{\rho_1 A_1}{\rho_2 A_2} - U_2 = 0 \ . \tag{5}$$

In order to manipulate (5) further, we must consider the ratios of the densities and the cross-sectional areas. When subjected to a small-displacement deformation  $\mathbf{u}=(u,v,w)$ , the volume V of a differential element initially (in the undeformed state) having volume  $V_0$  becomes

$$V = V_0 [1 + \nabla \cdot \mathbf{u}] \equiv V_0 [1 + \varepsilon_x + \varepsilon_y + \varepsilon_z] , \qquad (6)$$

as is known from e.g. the theory of elasticity. Here  $\varepsilon_x$ ,  $\varepsilon_y$  and  $\varepsilon_z$  are the axial strains with respect to x, y and z -directions. Because the total mass M of the differential element is conserved in the small deformation, it follows for the density  $\rho$  that

$$\rho \equiv \frac{M}{V} = \frac{1}{V_0} \frac{M}{V/V_0} = \frac{M}{V_0} \frac{1}{V/V_0} = \rho_0 \left[ 1 + \varepsilon_x + \varepsilon_y + \varepsilon_z \right]^{-1}, \tag{7}$$

by (6) and the definition  $\rho_0 \equiv M/V_0$ .

Let us assume that the material is subjected to pure axial stress. This induces an axial strain  $\varepsilon_x$ , and via the Poisson effect, also the strains  $\varepsilon_y$  and  $\varepsilon_z$  in the two orthogonal directions:

$$\varepsilon_y = -\nu_{12}\varepsilon_x \;, \quad \varepsilon_z = -\nu_{13}\varepsilon_x \;, \tag{8}$$

where  $\nu_{12}$  and  $\nu_{13}$  are the (orthotropic) Poisson ratios for stretching in the direction of the material axis 1, describing the resulting contraction on material axes 2 and 3, respectively. This purely elastic approximation neglects all viscous effects, but since a steady state is being considered, this is reasonable. The cross-sectional area of the web is

$$A = (1 + \varepsilon_z)h(1 + \varepsilon_y)b \approx bh(1 + \varepsilon_y + \varepsilon_z), \qquad (9)$$

where second-order small terms have been neglected. Combining (8) and (9), we have

$$A = bh(1 - (\nu_{12} + \nu_{13})\varepsilon_x) \equiv bh(1 - \nu_{1A}\varepsilon_x),$$
(10)

where the effective Poisson ratio for the change in cross-sectional area, when stretched along material axis 1, is defined as

$$\nu_{1A} \equiv \nu_{12} + \nu_{13} \ . \tag{11}$$

As was noted in the introduction, for paper materials, it is known (see e.g. the study by Stenberg and Fellers [65]) that  $\nu_{13} < 0$ , and that  $|\nu_{13}|$  may be as large as 3.0. Typically, we will thus have  $\nu_{1A} < 0$ : the cross-sectional area may actually increase under tension, because the thickness increases. Even though the thickness itself is typically small, it may undergo a large relative change, and therefore must be considered when calculating the area of the cross-section.

It also follows from (6), (8) and (11) that

$$V = V_0 [1 + (1 - \nu_{1A})\varepsilon_x] . {12}$$

It should be pointed out that if for some particular material  $\nu_{1A} = 1$ , then  $V = V_0$ ,

and such a material behaves incompressibly when stretched along material axis 1. The effective Poisson ratio contains the directional Poisson ratios  $\nu_{12}$  and  $\nu_{13}$ . The only requirement for incompressibility under uniaxial stretching is that the sum of  $\nu_{12}$  and  $\nu_{13}$  is unity; unlike the isotropic case, neither of them needs to be 0.5.

Furthermore, the values of  $\nu_{23}$ ,  $\nu_{21}$ ,  $\nu_{31}$  and  $\nu_{32}$  still remain free. Elastic compatibility is required, but this brings in additional free parameters, because elastic compatibility involves not only the Poisson ratios, but also the Young moduli; e.g.  $E_1\nu_{21}=E_2\nu_{12}$ . Depending on the set of parameter values chosen, it is possible that an orthotropic material behaves incompressibly in axial stretching only when the deformation is applied along some particular axis.

It is thus evident that the conditions of incompressibility for anisotropic materials are more complicated than for isotropic materials, where the only requirement is  $\nu=0.5$ . For a more thorough consideration of incompressibility in orthotropic and transversely isotropic materials, see the study by Itskov and Aksel [30].

By combining (7), (8) and (11), we obtain

$$\rho = \frac{\rho_0}{1 + (1 - \nu_{1A})\varepsilon_x} \ . \tag{13}$$

In the following, we shall assume that the material, subjected to constant axial tension at the rollers (traction lines), has zero strain at  $A_1$ , and experiences some nonzero axial strain  $\varepsilon_x$  at  $A_2$ , due to the applied axial stress. Preliminary one-dimensional results [36] indicate that such a strain state occurs at least for an axially travelling Kelvin–Voigt viscoelastic material; see also the treatment of the one-dimensional case further below, where we will show this briefly. By (10), the cross-sectional areas at the inflow and outflow surfaces  $A_1$  and  $A_2$  become

$$A_1 = bh , \qquad A_2 = bh(1 - \nu_{1A}\varepsilon_x) , \qquad (14)$$

and by (13), the material densities on these surfaces are

$$\rho_1 = \rho_0 , \qquad \rho_2 = \frac{\rho_0}{1 + (1 - \nu_{1A})\varepsilon_x} .$$
(15)

By inserting (14) and (15) into the mass balance (5), simplifying, and solving for  $\varepsilon_x$ , we obtain the result

$$\varepsilon_x = \frac{\frac{U_2}{U_1} - 1}{1 + \left[\frac{U_2}{U_1} - 1\right] \nu_{1A}} \,. \tag{16}$$

Equation (16) gives the axial strain, at the traction line at  $x = \ell$ , for the problem of in-plane (visco-)elastic deformation, corresponding to given roller speeds  $U_1$  and  $U_2$ . Obviously, in order for the model to remain valid, the given velocities must be such that the strain according to (16) remains in the small-deformation range.

The transport velocity of the flowing solid continuum in the above case is assumed to be controlled only in the x (longitudinal) direction; all in-plane deforma-

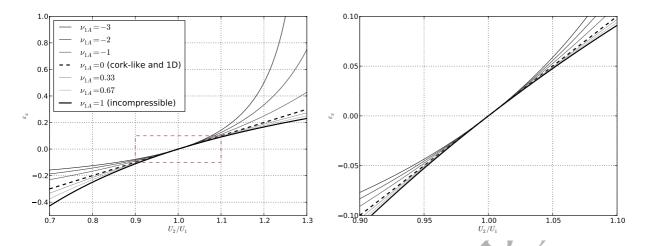


Figure 3: Roller-induced axial strain  $\varepsilon_x$  as a function of velocity ratio  $U_2/U_1$ , based on equations (16)–(18). Left: Overall behaviour of the functions, showing their curvature (only the line for  $\nu_{1A}=0$  is straight). Right: Zoomed-in view. Location of the zoomed area is shown by the dashed box in the left subfigure.

tions in the y (widthwise) direction are determined by the (visco-)elastic response. It should also be kept in mind that (16) only applies in a steady-state flow, i.e. when the web flows smoothly without time-dependent disturbances.

The traction lines at the cross-sectional areas  $A_1$  and  $A_2$  affect only the surfaces of the web, which implies that stress and strain waves advancing inside the web thickness can cross the traction lines. Therefore in the boundary conditions of moving continuous webs, in reality, one should consider rather complicated friction-based force transmission phenomena at web-roll contact areas [35].

In the special case of a material which behaves incompressibly when stretched along axis 1, we have  $\nu_{1A} = 1$ . By inserting this into (16) and simplifying, we obtain

$$\varepsilon_x = 1 - \frac{U_1}{U_2} \,, \tag{17}$$

which holds only when  $\nu_{1A} = 1$ .

In the limit  $\nu_{1A} \rightarrow 0$ , equation (16) simplifies to

$$\varepsilon_x = \frac{U_2}{U_1} - 1 \ . \tag{18}$$

The behaviour of equations (16)–(18) is illustrated in Figure 3.

Equation (18) corresponds to cork-like materials, which do not exhibit the Poisson effect. Note, however, that if the material is an auxetic orthotropic one, it is possible that  $\nu_{13} = -\nu_{12}$ , also leading to  $\nu_{1A} = 0$ .

The result (18) also arises in the case of a one-dimensional string model. Consider a compressible travelling string, undergoing steady-state flow through the span 0 <

 $x < \ell$ . Mass conservation now requires

$$R_1 U_1 - R_2 U_2 = 0 (19)$$

with similar definitions for the subscripts as above. Here R is the linear density,  $[R] = \mathrm{kg/m}$ , and the cross-sectional area of the string is assumed constant. As the string becomes stretched or compressed, the linear density changes as

$$R = \frac{R_0}{1 + \varepsilon_x} \,, \tag{20}$$

where  $R_0$  is the (constant) linear density in the initial (unstretched) state. Equation (20) follows directly from (7) and the assumption that the cross-sectional area is constant (whence  $\varepsilon_y = \varepsilon_z = 0$ ).

As before, let as assume that at x=0 the strain in the string is zero, and at  $x=\ell$ , the string experiences some nonzero strain  $\varepsilon_x$  due to axial tension applied at the ends of the span. As was noted above, this is consistent for a travelling Kelvin–Voigt viscoelastic string. Hence  $R_1=R_0$ , and  $R_2$  is given by (20). By combining (19) and (20), and solving for  $\varepsilon_x$ , the equation (18) is obtained.

#### 3. The governing equations

In this section we will define the stresses and strains, deformations, material assumptions and velocity-dependent in-plane inertial forces for the moving web. This leads to both one- and two-dimensional models, and equations for the viscoelastic moving web continuum application.

The standard approach for describing structural deformations is the Lagrangean description. However, longitudinal in-plane deformations in axially moving materials are more challenging. One possibility is to actually move the medium at the desired speed, and update the boundary conditions at each timestep [64]. Another possibility is to use an Eulerian flow description, and by this the actual deformation of the moving continuum can be handled using a mixed Lagrange–Euler description [34]. The Eulerian description is a standard approach in fluid dynamics where the observer is watching a control volume, where possible deformations will appear [64, 38].

In this paper, we consider two-dimensional in-plane membrane behaviour. Based on Figure 4, one can derive the following well-known force balance [69]:

$$\frac{\partial \sigma_x}{\partial x} + \frac{\partial \tau_{yx}}{\partial y} + F_x = 0 , \qquad (21)$$

$$\frac{\partial \tau_{xy}}{\partial x} + \frac{\partial \sigma_y}{\partial y} + F_y = 0 , \qquad (22)$$

where  $\tau_{yx}$  and  $\tau_{xy}$  are shear stresses and  $F_x$  and  $F_y$  are (external) body forces. The



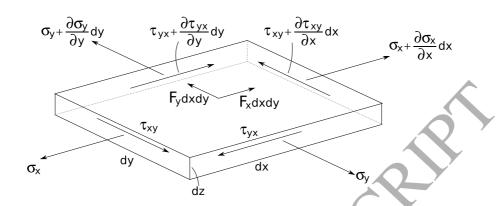


Figure 4: Differential parallelepiped and forces acting in the in-plane directions x and y.

linear Cauchy strains are

$$\varepsilon_{xx} = \frac{\partial u}{\partial x}, \ \varepsilon_{yy} = \frac{\partial v}{\partial y}, \ \varepsilon_{xy} = \frac{\partial u}{\partial y}, \ \varepsilon_{yx} = \frac{\partial v}{\partial x}, \ \gamma_{xy} = \left(\frac{\partial u}{\partial y} + \frac{\partial v}{\partial x}\right). \tag{23}$$

We will use the material assumption of orthotropic viscoelasticity. In the mixed Lagrange–Euler description, the strains

$$\varepsilon_{ij} = \varepsilon_{ij} (x, y, t)$$
 (24)

lead to the time derivatives

$$\frac{\mathrm{d}\varepsilon_{ij}}{\mathrm{d}t} = \frac{\partial\varepsilon_{ij}}{\partial t} + \frac{\partial\varepsilon_{ij}}{\partial x}\frac{\mathrm{d}x}{\mathrm{d}t} + \frac{\partial\varepsilon_{ij}}{\partial y}\frac{\mathrm{d}y}{\mathrm{d}t} = \frac{\partial\varepsilon_{ij}}{\partial t} + \frac{\partial\varepsilon_{ij}}{\partial x}U_x + \frac{\partial\varepsilon_{ij}}{\partial y}U_y , \qquad (25)$$

where  $(U_x, U_y)$  is the velocity field and d/dt is the material derivative.

With fibrous, composite-type materials, the mechanical deformation response properties are the result of complicated material preprocessing, which results in orthotropic anisotropy and material time-dependence (see e.g. [70, 32, 53, 8]). It is possible to derive a vast number of different rheological models for time-dependent material behaviour, but the fundamental behaviour of continuous flow of a solid viscoelastic web can be analyzed using the simple classical Kelvin–Voigt model. The singular stress response to a step strain does not matter, because we are considering a steady state where no rapidly changing strains occur; hence the Kelvin–Voigt model is sufficient for the present application.

The stress-strain behaviour of one-dimensional Kelvin-Voigt material (see e.g. [19]) is simply

$$\sigma = \mathbf{E}\varepsilon + \eta \frac{\mathrm{d}\varepsilon}{\mathrm{d}t} \,, \tag{26}$$

where E is Young's modulus and  $\eta$  is the material viscosity. We will apply a twodimensional, orthotropic plane stress extension of the above model, based on practical observations of fibrous web materials [68, 70]:

$$\sigma_{x} = \frac{E_{x}}{1 - \nu_{12}\nu_{21}} \left(\varepsilon_{xx} + \nu_{21}\varepsilon_{yy}\right) + \frac{\eta_{x}}{1 - \varphi_{12}\varphi_{21}} \left(\frac{d\varepsilon_{xx}}{dt} + \varphi_{21}\frac{d\varepsilon_{yy}}{dt}\right)$$
(27)

$$\sigma_{y} = \frac{E_{y}}{1 - \nu_{12}\nu_{21}} \left(\varepsilon_{yy} + \nu_{12}\varepsilon_{xx}\right) + \frac{\eta_{y}}{1 - \varphi_{12}\varphi_{21}} \left(\frac{d\varepsilon_{yy}}{dt} + \varphi_{12}\frac{d\varepsilon_{xx}}{dt}\right)$$
(28)

$$\tau_{yx} = \tau_{xy} = G\gamma_{xy} + \Pi \frac{d\gamma_{xy}}{dt}$$
 (29)

Here  $\varphi_{12}$  and  $\varphi_{21}$  are the viscous analogues of the orthotropic in-plane Poisson ratios  $\nu_{12}$  and  $\nu_{21}$ , G is a shear modulus,  $\gamma$  is a shear strain and  $\Pi$  is the viscous shear modulus.

Often, problems considering the in-plane behaviour of a continuous material are written using only an elastic model, involving the moduli of elasticity  $E_x$  and  $E_y$ , and the strain variables  $\varepsilon_{xx}$  and  $\varepsilon_{yy}$ . However, in practice all the elastic-related material properties are measured with some definite speed, and therefore apparent elasticity includes both elastic and viscous material properties [19]. Fundamentally, all materials exhibit some form of viscoelasticity, typically measured by normal and complex moduli E and E', respectively. It should also be pointed out that the viscoelastic Poisson ratios  $\varphi_{12}$  and  $\varphi_{21}$  cannot be calculated using compliances from the theory of elasticity [25].

In fibrous cellulose-based materials, moisture-dependent dimension changes can be significant, which generates stresses in addition to those related to the strains based on external velocity differences (equation (16)). In this study, we have chosen not to model these moisture-dependent stresses.

Let us now consider the dynamic equilibrium. According to Newton's second law, time-dependent dynamical behaviour always includes inertial forces. Two-dimensional inertial forces in in-plane membrane behaviour can be accounted for using the following dynamic equilibrium equations (see e.g. [60]):

$$\frac{\partial \sigma_x}{\partial x} + \frac{\partial \tau_{xy}}{\partial y} + F_x = \rho \frac{\mathrm{d}^2 u}{\mathrm{d}t^2} \tag{30}$$

$$\frac{\partial \sigma_y}{\partial y} + \frac{\partial \tau_{yx}}{\partial x} + F_y = \rho \frac{\mathrm{d}^2 v}{\mathrm{d}t^2} \tag{31}$$

Note that the operator  $d^2/dt^2$  describes the inertial behaviour in the Lagrangean reference frame. Thus the inertial terms depending on the displacements u and v, in

the Eulerian frame, must be presented using the material derivative:

$$\frac{\mathrm{d}u}{\mathrm{d}t} = \frac{\partial u}{\partial t} + \sum_{i=1,2} \frac{\partial u}{\partial x_i} \frac{\mathrm{d}x_i}{\mathrm{d}t}$$
(32)

$$\frac{\mathrm{d}v}{\mathrm{d}t} = \frac{\partial v}{\partial t} + \sum_{i=1,2} \frac{\partial v}{\partial x_i} \frac{\mathrm{d}x_i}{\mathrm{d}t}$$
(33)

The second material derivatives of the displacements u and v are

$$\frac{d^{2}u}{dt^{2}} = \frac{\partial^{2}u}{\partial t^{2}} + 2U_{x}\frac{\partial^{2}u}{\partial x\partial t} + 2U_{y}\frac{\partial^{2}u}{\partial y\partial t} + U_{x}^{2}\frac{\partial^{2}u}{\partial x^{2}} + 2U_{x}U_{y}\frac{\partial^{2}u}{\partial x\partial y} + U_{y}^{2}\frac{\partial^{2}u}{\partial y^{2}}$$

$$+ \frac{\partial u}{\partial x}\left(\frac{\partial U_{x}}{\partial x}U_{x} + \frac{\partial U_{x}}{\partial y}U_{y} + \frac{\partial U_{x}}{\partial t}\right) + \frac{\partial u}{\partial y}\left(\frac{\partial U_{y}}{\partial x}U_{x} + \frac{\partial U_{y}}{\partial y}U_{y} + \frac{\partial U_{y}}{\partial t}\right)$$
(34)

$$\frac{d^{2}v}{dt^{2}} = \frac{\partial^{2}v}{\partial t^{2}} + 2U_{x}\frac{\partial^{2}v}{\partial x\partial t} + 2U_{y}\frac{\partial^{2}v}{\partial y\partial t} + U_{x}^{2}\frac{\partial^{2}v}{\partial x^{2}} + 2U_{x}U_{y}\frac{\partial^{2}v}{\partial x\partial y} + U_{y}^{2}\frac{\partial^{2}v}{\partial y^{2}} + \frac{\partial^{2}v}{\partial y^{2}} + \frac{\partial^{2}v}{\partial x}U_{x} + \frac{\partial^{2}v}{\partial x}U_{x} + \frac{\partial^{2}v}{\partial y}U_{y} + \frac{\partial^{2}v}{\partial y}U_{y} + \frac{\partial^{2}v}{\partial y}U_{x} + \frac{\partial^{2}v}{\partial y}U_{y} + \frac{\partial^{2}v}{\partial$$

By substituting (34)–(35) and (27)–(29) into (30) and (31), we obtain the following time-dependent two-dimensional equations for the in-plane, plane stress membrane behaviour of the moving viscoelastic web:

$$(C_{11} - \rho U_x^2) \frac{\partial^2 u}{\partial x^2} + \left(C_{66} - \rho U_y^2\right) \frac{\partial^2 u}{\partial y^2} + C_{12} \frac{\partial^2 v}{\partial x \partial y} + C_{66} \frac{\partial^2 v}{\partial x \partial y} + K_{11} U_x \frac{\partial^3 u}{\partial x^3}$$

$$+ K_{11} U_y \frac{\partial^3 u}{\partial x^2 \partial y} + K_{11} \frac{\partial^3 u}{\partial x^2 \partial t} + (K_{12} + K_{66}) U_y \frac{\partial^3 v}{\partial x \partial y^2} + K_{66} U_y \frac{\partial^3 u}{\partial y^3}$$

$$+ (K_{12} + K_{66}) U_x \frac{\partial^3 v}{\partial x^2 \partial y} + (K_{12} + K_{66}) \frac{\partial^3 v}{\partial x \partial y \partial t} + K_{66} U_x \frac{\partial^3 u}{\partial x \partial y^2} + K_{66} \frac{\partial^3 u}{\partial y^2 \partial t} + F_x$$

$$= \rho \frac{\partial^2 u}{\partial t^2} + 2\rho U_x U_y \frac{\partial^2 u}{\partial x \partial y} + 2\rho U_x \frac{\partial^2 u}{\partial x \partial t} + 2\rho U_y \frac{\partial^2 u}{\partial y \partial t}$$

$$+ \rho \frac{\partial u}{\partial y} \left( \frac{\partial U_x}{\partial x} U_x + \frac{\partial U_x}{\partial y} U_y + \frac{\partial U_x}{\partial t} \right) + \rho \frac{\partial u}{\partial y} \left( \frac{\partial U_y}{\partial x} U_x + \frac{\partial U_y}{\partial y} U_y + \frac{\partial U_y}{\partial t} \right)$$

$$(36)$$

and

$$\left(C_{22} - \rho U_y^2\right) \frac{\partial^2 v}{\partial y^2} + \left(C_{66} - \rho U_x^2\right) \frac{\partial^2 v}{\partial x^2} + C_{21} \frac{\partial^2 u}{\partial x \partial y} + C_{66} \frac{\partial^2 v}{\partial x \partial y} + K_{22} U_y \frac{\partial^3 v}{\partial y^3} 
+ K_{22} U_x \frac{\partial^3 v}{\partial x \partial y}^2 + K_{22} \frac{\partial^3 v}{\partial x^2 \partial t} + \left(K_{21} + K_{66}\right) U_x \frac{\partial^3 u}{\partial x^2 \partial y} + K_{66} U_x \frac{\partial^3 v}{\partial x^3} 
+ \left(K_{21} + K_{66}\right) U_y \frac{\partial^3 u}{\partial x \partial y^2} + \left(K_{21} + K_{66}\right) \frac{\partial^3 u}{\partial x \partial y \partial t} + K_{66} U_y \frac{\partial^3 v}{\partial x^2 \partial y} + K_{66} \frac{\partial^3 v}{\partial y^2 \partial t} + F_y 
= \rho \frac{\partial^2 v}{\partial t^2} + 2\rho U_x U_y \frac{\partial^2 v}{\partial x \partial y} + 2\rho U_x \frac{\partial^2 v}{\partial x \partial t} + 2\rho U_y \frac{\partial^2 v}{\partial y \partial t} 
+ \rho \frac{\partial v}{\partial x} \left(\frac{\partial U_x}{\partial x} U_x + \frac{\partial U_x}{\partial y} U_y + \frac{\partial U_x}{\partial t}\right) + \rho \frac{\partial v}{\partial y} \left(\frac{\partial U_y}{\partial x} U_x + \frac{\partial U_y}{\partial y} U_y + \frac{\partial U_y}{\partial t}\right) ,$$
(37)

where the coefficients are

$$C_{11} = \frac{E_x}{1 - \nu_{12}\nu_{21}}, \qquad C_{22} = \frac{E_y}{1 - \nu_{12}\nu_{21}}, \qquad (38)$$

$$C_{12} = C_{11}\nu_{21} = C_{22}\nu_{12} = C_{21}, \qquad C_{66} = G, \qquad (39)$$

$$K_{11} = \frac{\eta_x}{1 - \varphi_{12}\varphi_{21}}, \qquad K_{22} = \frac{\eta_y}{1 - \varphi_{12}\varphi_{21}}, \qquad (40)$$

$$K_{12} = K_{11}\varphi_{21} = K_{22}\varphi_{12} = K_{21}, \quad K_{66} = \Pi. \qquad (41)$$

$$C_{12} = C_{11}\nu_{21} = C_{22}\nu_{12} = C_{21}, \quad C_{66} = G,$$
 (39)

$$K_{11} = \frac{\eta_x}{1 - \varphi_{12}\varphi_{21}}, \qquad K_{22} = \frac{\eta_y}{1 - \varphi_{12}\varphi_{21}}, \qquad (40)$$

$$K_{12} = K_{11}\varphi_{21} = K_{22}\varphi_{12} = K_{21}, \quad K_{66} = \Pi.$$
 (41)

The equations (36) and (37) are nonlinear. Nonlinearity appears in the velocities  $U_x$ and  $U_y$ , which are dependent on the deformation. There are also nonlinear Navier– Stokes type convection terms, the significance of which is small if the strains defined in (23) can be considered small.

#### 4. Steady state of one-dimensional viscoelastic moving continuum

One-dimensional application of (36) and (37) will lead to the following timedependent solid continuum flow equation [36]:

$$\rho \frac{\partial^{2} u}{\partial t^{2}} + 2\rho U_{x} \frac{\partial^{2} u}{\partial x \partial t} + \rho U_{x}^{2} \frac{\partial^{2} u}{\partial x^{2}} + \rho \frac{\partial u}{\partial x} \left( \frac{\partial U_{x}}{\partial x} U_{x} + \frac{\partial U_{x}}{\partial t} \right)$$

$$= E_{x} \frac{\partial^{2} u}{\partial x^{2}} + \eta_{x} \left( \frac{\partial^{3} u}{\partial x^{2} \partial t} + U_{x} \frac{\partial^{3} u}{\partial x^{3}} \right)$$

$$(42)$$

In a steady state, where there is no time-dependent fluctuation in the displacement, we can write the following equation for ideal, undisturbed axial narrow web flow:

$$\eta_x U_x \frac{\partial^3 u}{\partial x^3} + \left( E_x - \rho U_x^2 \right) \frac{\partial^2 u}{\partial x^2} - \rho \frac{\partial u}{\partial x} \left( \frac{\partial U_x}{\partial x} U_x \right) = 0.$$
 (43)

In general, equation (43) is nonlinear due to the dependence of  $U_x$  on  $\partial u/\partial x$ . To see how this arises in the one-dimensional case, recall (16) and its one-dimensional specialization (18), which were obtained via a mass conservation argument. There are now two possibilities. The strain  $\varepsilon_{xx} = \partial u/\partial x$  (equation (23)) either depends on x, or is constant along the span.

Let us first investigate the case where the strain depends on x. This was a starting assumption in the derivation of (18); if it holds, then also (18) holds. Let the velocity  $U_1$  at the beginning of the span be fixed. By rearranging, (18) becomes

$$U_2 = U_1[1 + \varepsilon_{xx}] , \qquad (44)$$

where we now use the notation  $\varepsilon_{xx}$  for the strain.

Because mass conservation must hold for any value of x, equation (44) is valid along the whole span, and we have  $U_x(x)=U_2(x)$ . However, because  $\varepsilon_{xx}=\varepsilon_{xx}(x)=(\partial u/\partial x)(x)$  appears on the right-hand side of (44), actually  $U_2=U_2(\varepsilon_{xx})$ . Thus, if the strain varies along the span, mass conservation implies that a linear relationship exists between the strain and velocity fields.

Because  $U_x$  is now a linear function of  $\varepsilon_{xx}$ , all terms of (43) involving factors of  $U_x$  and its derivatives become nonlinear with respect to the unknown u and its derivatives. Using  $U_2$  from (44) as  $U_x$  in (43), we see that the nonlinearity is of a polynomial form; up to cubic terms are present. Numerical FEM solution of the nonlinear equation has been presented in [36].

The other possibility is that the strain is a constant along the whole span, in which case the starting assumption leading to (18) does not hold. On the other hand, a constant value for  $\varepsilon_{xx} = \partial u/\partial x$  implies  $\partial^2 u/\partial x^2 = \partial^3 u/\partial x^3 \equiv 0$ . Let us investigate the nontrivial case where  $\varepsilon_{xx} \neq 0$ . The first and second terms in (43) vanish. Dividing by the nonzero constant strain  $\partial u/\partial x$  and the material density  $\rho \neq 0$ , all that remains is

$$\frac{\partial U_x}{\partial x}U_x = 0 \ . \tag{45}$$

Equation (45) must hold pointwise at any x. Thus for each fixed x, either  $\partial U_x/\partial x=0$  or  $U_x=0$ . More interesting of the two is  $\partial U_x/\partial x=0$ , which leads to  $U_x=0$  constant.

We conclude that if a constant strain is observed, this implies that the velocity must also be constant. However, the reverse is not true, as we will see below. The value of the constant strain is determined by the boundary conditions, which were not analyzed here.

For the rest of this section, we again let  $\varepsilon_{xx}$  vary along the span, concentrating on

the special case where  $U_x$  is approximately constant:

$$U_x(x) = U_0 + \phi(x) . \tag{46}$$

Here  $U_0$  is a constant, and the arbitrary function  $\phi(x)$  and its derivatives are considered small. If the strain variable and its derivatives are small, and the velocity field is of the form (44), this representation is applicable. Inserting (46) into (43) and dropping second-order small terms leads to a linear equation, namely

$$\eta_x U_0 \frac{\partial^2 \varepsilon_{xx}}{\partial x^2} + \left( E_x - \rho U_0^2 \right) \frac{\partial \varepsilon_{xx}}{\partial x} = 0 , \qquad (47)$$

which is of the form of a standard linear steady-state advection-diffusion equation.

To obtain this form, it is not sufficient that  $\phi$  itself is small; also  $\partial \phi/\partial x$  must be small to avoid an additional term  $-\rho \varepsilon_{xx} [\partial \phi/\partial x] U_0$  on the left-hand side. Observe that (47) is exact in the case where  $U_x$  is (an exact) constant. Thus any conclusions will apply to cases with exactly constant as well as approximately constant transport velocities.

The equation (47) can be solved using analytical methods. First, as a special case, if only pure elasticity is present ( $\eta_x = 0$ ), the solution is of the form [36]

$$\left(E_x - \rho U_0^2\right)\varepsilon_{xx} = C. \tag{48}$$

In this case the solution describes Hookean behaviour, i.e., the strain  $\varepsilon_{xx}$  is constant along the whole span regardless of the magnitude of the constant transport velocity  $U_0$ . The corresponding stress field, from (26) with  $\eta_x = 0$  and (48), is also constant:

$$\sigma_x = E_x \varepsilon_{xx} = \frac{E_x C}{(E_x - \rho U_0^2)} \equiv C' . \tag{49}$$

The values of the constants C and C' are determined by the boundary conditions. Because (48) is a first-order ordinary differential equation with respect to  $\varepsilon_{xx}$ , for this variable we may set only one boundary condition. Choosing this boundary condition as  $\varepsilon_{xx}(\ell) = \varepsilon_T$ , from (48) we have  $C = (E_x - \rho U_0^2) \varepsilon_T$ , which leads to  $\varepsilon_{xx}(x) = \varepsilon_T$  (for all x), and for the stress  $\sigma_x = E_x \varepsilon_T$ . This may seem trivial, but it provides an important point of comparison for the following case.

Consider a material with viscosity  $\eta_x \neq 0$ , moving axially at a velocity  $U_0 \neq 0$ . Equation (47) becomes, after division by  $\eta_x U_0$ ,

$$\frac{\partial^2 \varepsilon_{xx}}{\partial x^2} + \left(\frac{E_x - \rho U_0^2}{\eta_x U_0}\right) \frac{\partial \varepsilon_{xx}}{\partial x} = 0.$$
 (50)

The differential equation is of the second order in  $\varepsilon_{xx}$ , hence two boundary conditions are required. With the choices  $\varepsilon_{xx}(0) = 0$  and  $\varepsilon_{xx}(\ell) = \varepsilon_T$ , the analytical

solution of (50) is [35, 37]:

$$\varepsilon_{xx}(x) = \varepsilon_T \frac{1 - e^{-kx}}{1 - e^{-k\ell}}, \quad \text{where} \quad k = \frac{E_x - \rho U_0^2}{\eta_x U_0}.$$
(51)

We have thus obtained the following result: for a Kelvin–Voigt viscoelastic material moving at a constant or an approximately constant transport velocity, loaded only by a prescribed strain at the ends of the span, the strain follows a saturating exponential curve along the span. This is unlike the case of purely elastic material, where the strain along the span is constant.

As  $x \to \ell$ , the strain approaches the same value it had in the elastic case,  $\varepsilon_T$ . Additionally, if  $\ell$  is large, the exponential terms approximately vanish in most of the domain, and the strain approximately obtains its elastic value everywhere except in a short boundary layer at the start of the span. The larger k is, the shorter is the boundary layer. Especially, the boundary layer vanishes in the limits  $\eta_x \to 0$  and  $U_0 \to 0$ , where  $k \to \infty$  (see definition of k in equation (51)). This agrees with the elastic solution.

The physical conclusion is that the strain  $\varepsilon_{xx}$  depends on x only if both the viscosity  $\eta_x$  and the transport velocity  $U_0$  are nonzero. In other words, in the context of this analysis, the effect appears only if the material is both viscoelastic and subjected to axial motion.

Finally, let us find the corresponding stress field. The stress, which is based on the strain in (51), is a superposition of elastic and viscous stress components ([36]; see also equation (26)):

$$\sigma_x = E_x \varepsilon_{xx} + \eta_x \frac{\mathrm{d}\varepsilon_{xx}}{\mathrm{d}t} \ . \tag{52}$$

Straightforward analytical solution of the stress is easy to obtain only in the linearized, one-dimensional, steady-state case having constant transport velocity [36]. Using (24) and (25), the time derivative of the strain, in mixed Lagrange–Euler form, can be written for the steady-state solution as follows:

$$\frac{\mathrm{d}\varepsilon_{xx}}{\mathrm{d}t} = \frac{\partial\varepsilon_{xx}}{\partial x}U_x \ . \tag{53}$$

Inserting (53) into (52), using (46) for  $U_x$ , noting that both  $\partial \varepsilon_{xx}/\partial x$  and  $\phi(x)$  are small, and then using the analytical solution (51), we obtain the stress field as

$$\sigma_x = E_x \varepsilon_T \frac{1 - e^{-kx}}{1 - e^{-k\ell}} + \eta_x U_0 \varepsilon_T \frac{ke^{-kx}}{1 - e^{-k\ell}}.$$
 (54)

where the elastic and viscous contributions are apparent. Both contributions follow an exponential curve. By expanding the multiplicative factor k in the second term of (54) (using again (51)), the result simplifies to

$$\sigma_x = \frac{\varepsilon_T}{1 - e^{-k\ell}} \left( E_x - \rho U_0^2 e^{-kx} \right), \tag{55}$$

which shows the total stress more clearly. Equation (55) is valid for  $\eta_x \neq 0$ ,  $U_0 \neq 0$ . We observe that also the total stress follows an exponential curve.

For large  $\ell$ , the exponentials again approximately vanish in most of the domain, and the stress approximately obtains its elastic value, except in a short boundary layer at the start of the span. The previous observations regarding k apply also here.

In this section, we have seen that for a narrow strip of Kelvin–Voigt material moving at a constant or an approximately constant transport velocity, both the strain and stress fields follow an exponential shape along the span. This effect appears only if the material is both viscoelastic and subjected to axial motion.

#### 5. The weak form and the natural boundary conditions

In the following, we will concentrate on the two-dimensional, steady-state, purely elastic case. This is obtained from (36)–(37) by omitting time-dependent terms and setting the viscous coefficients to zero.

The aim in the rest of this paper is to numerically study the two-dimensional, orthotropic, elastic moving continuum plane stress problem using the finite element method (FEM). The above approach was extremely useful for the one-dimensional analysis that was performed, but it is difficult to correctly derive the weak form by starting from (36)–(37).

The difficulty arises because the equations contain terms with mixed derivatives,  $\partial^2 u/\partial x \partial y$  and  $\partial^2 v/\partial x \partial y$ . It is not immediately clear how these terms should be considered when applying integration by parts in the component-form equations. Each such mixed term can be integrated by parts in either the x or the y direction, and each such choice will produce different contributions to the natural boundary conditions of the weak problem. Most combinations of choices lead to boundary conditions which make no physical sense, and the correct combination of choices is not obvious.

Hence, we will derive the weak form in cartesian tensor notation. Although in the present study only a rectangular sheet will be studied, this approach provides the additional advantage of arbitrary domain shape. We start with the general dynamic equilibrium equations (30)–(31), rewritten in tensor notation, and then expand the second material derivative. The dynamic equilibrium reads

$$\rho \frac{\mathrm{d}^2 \mathbf{u}}{\mathrm{d}t^2} - \nabla \cdot \boldsymbol{\sigma}^{\mathrm{T}} = \mathbf{F} , \qquad (56)$$

where  $(...)^T$  denotes the transpose of a rank-2 tensor. In the case where U is not time-dependent, the second material derivative expands as

$$\frac{d^2\mathbf{u}}{dt^2} = \frac{d}{dt} \left( \frac{\partial \mathbf{u}}{\partial t} + \mathbf{U} \cdot \nabla \mathbf{u} \right) = \frac{\partial^2 \mathbf{u}}{\partial t^2} + 2\mathbf{U} \cdot \nabla \left( \frac{\partial \mathbf{u}}{\partial t} \right) + (\mathbf{U} \cdot \nabla)(\mathbf{U} \cdot \nabla \mathbf{u}) . \tag{57}$$

Applying (57), the steady-state form of equation (56) becomes

$$\rho \left( \mathbf{U} \cdot \nabla \right) \left( \mathbf{U} \cdot \nabla \mathbf{u} \right) - \nabla \cdot \boldsymbol{\sigma}^{\mathrm{T}} = \mathbf{F} . \tag{58}$$

Next, we will use the principle of virtual work. Let us take the dot product of the equation (58) with a vector-valued test function (virtual displacement)  $\phi$ , and integrate the equation over the two-dimensional domain

$$\Omega \equiv \{ (x, y) : 0 < x < \ell, \ 0 < y < b \}.$$
 (59)

We assume that the material density  $\rho$  can be approximated as a constant and obtain

$$\rho \int_{\Omega} \boldsymbol{\phi} \cdot (\mathbf{U} \cdot \nabla)(\mathbf{U} \cdot \nabla \mathbf{u}) \, d\Omega - \int_{\Omega} \boldsymbol{\phi} \cdot [\nabla \cdot \boldsymbol{\sigma}^{\mathrm{T}}] \, d\Omega = \int_{\Omega} \boldsymbol{\phi} \cdot \mathbf{F} \, d\Omega . \tag{60}$$

In order to integrate by parts in (60), we make use of the following two identities:

$$\int_{\Omega} \boldsymbol{\phi} \cdot [\nabla \cdot \boldsymbol{\sigma}^{\mathrm{T}}] \, d\Omega = \int_{\partial \Omega} \boldsymbol{\phi} \cdot (\mathbf{n} \cdot \boldsymbol{\sigma}^{\mathrm{T}}) \, d\Gamma - \int_{\Omega} \nabla \boldsymbol{\phi} : \boldsymbol{\sigma} \, d\Omega , \qquad (61)$$

$$\int_{\Omega} \boldsymbol{\phi} \cdot (\mathbf{U} \cdot \nabla) (\mathbf{U} \cdot \nabla \mathbf{u}) \, d\Omega = \int_{\partial \Omega} \mathbf{n} \cdot [(\mathbf{U} \cdot \nabla \mathbf{u}) \cdot (\boldsymbol{\phi} \otimes \mathbf{U})] \, d\Gamma 
- \int_{\Omega} (\mathbf{U} \cdot \nabla \mathbf{u}) \cdot [\nabla \cdot (\mathbf{U} \otimes \boldsymbol{\phi})] \, d\Omega .$$
(62)

where n is the outer unit normal, and the notational conventions are

$$(\nabla \mathbf{a})_{ij} \equiv \partial_i a_j \,, \quad (\nabla \cdot \mathbf{A})_j \equiv \partial_i A_{ij} \quad (\mathbf{a} \otimes \mathbf{b})_{ij} = a_i b_j \,, \quad \mathbf{A} : \mathbf{B} \equiv A_{ij} B_{ji} \,.$$
 (63)

Here a and b are vectors, and A and B are rank-2 tensors. The summation convention for repeated indices applies. Note the ordering of indices in the gradient.

The integration-by-parts formula (61) is standard in the theory of elasticity; only the formula (62) requires explanation. Observe that for any differentiable vector fields a, b and c,

$$\nabla \cdot (\mathbf{a} \cdot (\mathbf{b} \otimes \mathbf{c})) = \mathbf{b} \cdot (\mathbf{c} \cdot \nabla \mathbf{a}) + \mathbf{a} \cdot (\nabla \cdot (\mathbf{c} \otimes \mathbf{b})). \tag{64}$$

By integrating (64) over the domain  $\Omega$ , applying the divergence theorem to the left-hand side, and choosing  $\mathbf{a} = (\mathbf{U} \cdot \nabla \mathbf{u})$ ,  $\mathbf{b} = \phi$ , and  $\mathbf{c} = \mathbf{U}$ , relation (62) follows.

Using (61) and (62) in (60), we obtain

$$-\rho \int_{\Omega} (\mathbf{U} \cdot \nabla \mathbf{u}) \cdot [\nabla \cdot (\mathbf{U} \otimes \boldsymbol{\phi})] \, d\Omega + \int_{\Omega} \nabla \boldsymbol{\phi} : \boldsymbol{\sigma} \, d\Omega$$
$$+\rho \int_{\partial\Omega} \mathbf{n} \cdot [(\mathbf{U} \cdot \nabla \mathbf{u}) \cdot (\boldsymbol{\phi} \otimes \mathbf{U})] \, d\Gamma - \int_{\partial\Omega} \boldsymbol{\phi} \cdot (\mathbf{n} \cdot \boldsymbol{\sigma}^{\mathrm{T}}) \, d\Gamma = \int_{\Omega} \boldsymbol{\phi} \cdot \mathbf{F} \, d\Omega . \tag{65}$$

By using the identity

$$\nabla \cdot (\mathbf{U} \otimes \phi) = (\nabla \cdot \mathbf{U})\phi + \mathbf{U} \cdot \nabla \phi \tag{66}$$

and rearranging

$$\phi \cdot (\mathbf{n} \cdot \boldsymbol{\sigma}^{\mathrm{T}}) = (\mathbf{n} \cdot \boldsymbol{\sigma}^{\mathrm{T}}) \cdot \phi = \mathbf{n} \cdot (\boldsymbol{\sigma}^{\mathrm{T}} \cdot \phi) , \qquad (67)$$

we have

$$-\rho \int_{\Omega} (\mathbf{U} \cdot \nabla \mathbf{u}) \cdot [(\nabla \cdot \mathbf{U})\phi + \mathbf{U} \cdot \nabla \phi] \, d\Omega + \int_{\Omega} \nabla \phi : \boldsymbol{\sigma} \, d\Omega$$
$$+ \int_{\partial \Omega} \mathbf{n} \cdot [\rho (\mathbf{U} \cdot \nabla \mathbf{u}) \cdot (\boldsymbol{\phi} \otimes \mathbf{U}) - \boldsymbol{\sigma}^{\mathrm{T}} \cdot \boldsymbol{\phi}] \, d\Gamma = \int_{\Omega} \boldsymbol{\phi} \cdot \mathbf{F} \, d\Omega . \tag{68}$$

We may simplify (68) with

$$(\mathbf{U}\cdot\nabla\boldsymbol{\phi})\cdot(\mathbf{U}\cdot\nabla\mathbf{u}) = U_i(\partial_i\phi_j)U_k(\partial_ku_j) = (\partial_i\phi_j)(U_iU_k\partial_ku_j) = \nabla\boldsymbol{\phi}: (\mathbf{U}\otimes\mathbf{U}\cdot\nabla\mathbf{u})^{\mathrm{T}}, (69)$$

which allows us to combine some terms on the first line:

$$-\rho \int_{\Omega} \boldsymbol{\phi} \cdot (\mathbf{U} \cdot \nabla \mathbf{u}) (\nabla \cdot \mathbf{U}) \, d\Omega + \int_{\Omega} \nabla \boldsymbol{\phi} : \left[ \boldsymbol{\sigma} - \rho \left( \mathbf{U} \otimes \mathbf{U} \cdot \nabla \mathbf{u} \right)^{\mathrm{T}} \right] d\Omega$$
$$+ \int_{\partial \Omega} \mathbf{n} \cdot \left[ \rho \left( \mathbf{U} \cdot \nabla \mathbf{u} \right) \cdot (\boldsymbol{\phi} \otimes \mathbf{U}) - \boldsymbol{\sigma}^{\mathrm{T}} \cdot \boldsymbol{\phi} \right] d\Gamma = \int_{\Omega} \boldsymbol{\phi} \cdot \mathbf{F} \, d\Omega . \tag{70}$$

Finally, observing that

$$\mathbf{n} \cdot \left[ (\mathbf{U} \cdot \nabla \mathbf{u}) \cdot (\boldsymbol{\phi} \otimes \mathbf{U}) \right] = (\mathbf{U} \cdot \nabla \mathbf{u}) \cdot (\boldsymbol{\phi} \otimes \mathbf{U}) \cdot \mathbf{n} = \left( (\mathbf{U} \cdot \nabla \mathbf{u}) \cdot \boldsymbol{\phi} \right) (\mathbf{U} \cdot \mathbf{n})$$

$$= (\mathbf{n} \cdot \mathbf{U}) \left( (\mathbf{U} \cdot \nabla \mathbf{u}) \cdot \boldsymbol{\phi} \right) = (n_i U_i) \left( (U_j \partial_j u_k) \phi_k \right) = n_i \left( U_i U_j \partial_j u_k \right) \phi_k$$

$$= \mathbf{n} \cdot \left( \mathbf{U} \otimes \mathbf{U} \cdot \nabla \mathbf{u} \right) \cdot \boldsymbol{\phi} = \boldsymbol{\phi} \cdot \left( \mathbf{U} \otimes \mathbf{U} \cdot \nabla \mathbf{u} \right)^{\mathrm{T}} \cdot \mathbf{n}$$
obtain the result

we obtain the result

in the result
$$-\rho \int_{\Omega} \boldsymbol{\phi} \cdot (\mathbf{U} \cdot \nabla \mathbf{u}) (\nabla \cdot \mathbf{U}) \, d\Omega + \int_{\Omega} \nabla \boldsymbol{\phi} : \left[ \boldsymbol{\sigma} - \rho \left( \mathbf{U} \otimes \mathbf{U} \cdot \nabla \mathbf{u} \right)^{\mathrm{T}} \right] \, d\Omega$$

$$+ \int_{\partial \Omega} \boldsymbol{\phi} \cdot \left[ -\boldsymbol{\sigma} + \rho \left( \mathbf{U} \otimes \mathbf{U} \cdot \nabla \mathbf{u} \right)^{\mathrm{T}} \right] \cdot \mathbf{n} \, d\Gamma = \int_{\Omega} \boldsymbol{\phi} \cdot \mathbf{F} \, d\Omega . \tag{72}$$

Equation (72) holds for the steady-state in-plane equilibrium of any sheet of travelling material, as long as  $\rho$  is approximately constant. We will apply the material model later.

We see from (72) that if  $\nabla \cdot \mathbf{U} = 0$ , then the first term vanishes. In such a case, the equations have the same form as those of a stationary membrane, but now the stresses obtain contributions (both inside the domain and on the traction boundaries) from the additional term involving velocity. However, if the velocity field has nonzero divergence, then no such analogy can be drawn.

The boundary term of (72) gives the natural boundary conditions for this problem. The natural quantity to be prescribed is the normal component of the following rank-2 tensor:

$$\sigma_{\text{eff}} \equiv \sigma - \rho \left( \mathbf{U} \otimes \mathbf{U} \cdot \nabla \mathbf{u} \right)^{\text{T}}.$$
 (73)

This can be interpreted as an effective stress tensor, where the apparent stress (in laboratory coordinates) is modified by the centrifugal inertial effect.

#### 6. Component form of the weak form

For convenience of software implementation, let us split (72) into component form. Let us denote the components of the virtual displacement as  $\phi \equiv (\phi, \psi)$ . In the two-dimensional case being investigated in the present study, (72) gives the two equations

$$\int_{\Omega} \left[ -\sigma_{x} + \rho U_{x}^{2} \frac{\partial u}{\partial x} + \rho U_{y} U_{x} \frac{\partial u}{\partial y} \right] \frac{\partial \phi}{\partial x} d\Omega 
+ \int_{\Omega} \left[ -\tau_{xy} + \rho U_{x} U_{y} \frac{\partial u}{\partial x} + \rho U_{y}^{2} \frac{\partial u}{\partial y} \right] \frac{\partial \phi}{\partial y} d\Omega 
+ \rho \int_{\Omega} \left\{ \left[ U_{x} \frac{\partial u}{\partial x} + U_{y} \frac{\partial u}{\partial y} \right] \left[ \frac{\partial U_{x}}{\partial x} + \frac{\partial U_{y}}{\partial y} \right] \right\} \phi d\Omega 
+ \int_{\partial\Omega} n_{x} \left[ \sigma_{x} - \rho U_{x}^{2} \frac{\partial u}{\partial x} - \rho U_{y} U_{x} \frac{\partial u}{\partial y} \right] \phi d\Gamma 
+ \int_{\partial\Omega} n_{y} \left[ \tau_{xy} - \rho U_{x} U_{y} \frac{\partial u}{\partial x} - \rho U_{y}^{2} \frac{\partial u}{\partial y} \right] \phi d\Gamma 
+ \int_{\Omega} F_{x} \phi = 0 .$$
(74)

and

$$\int_{\Omega} \left[ -\tau_{yx} + \rho U_{x}^{2} \frac{\partial v}{\partial x} + \rho U_{y} U_{x} \frac{\partial v}{\partial y} \right] \frac{\partial \psi}{\partial x} d\Omega 
+ \int_{\Omega} \left[ -\sigma_{y} + \rho U_{x} U_{y} \frac{\partial v}{\partial x} + \rho U_{y}^{2} \frac{\partial v}{\partial y} \right] \frac{\partial \psi}{\partial y} d\Omega 
+ \rho \int_{\Omega} \left\{ \left[ U_{x} \frac{\partial v}{\partial x} + U_{y} \frac{\partial v}{\partial y} \right] \left[ \frac{\partial U_{x}}{\partial x} + \frac{\partial U_{y}}{\partial y} \right] \right\} \psi d\Omega 
+ \int_{\partial \Omega} n_{x} \left[ \tau_{yx} - \rho U_{x}^{2} \frac{\partial v}{\partial x} - \rho U_{y} U_{x} \frac{\partial v}{\partial y} \right] \psi d\Gamma 
+ \int_{\partial \Omega} n_{y} \left[ \sigma_{y} - \rho U_{x} U_{y} \frac{\partial v}{\partial x} - \rho U_{y}^{2} \frac{\partial v}{\partial y} \right] \psi d\Gamma 
+ \int_{\Omega} F_{y} \psi = 0 ,$$
(75)

where the outer unit normal **n** has components  $\mathbf{n} = (n_x, n_y)$ . We have moved all terms to the left-hand side and multiplied each equation by -1.

Equations (74)–(75) represent the steady state in-plane equilibrium for any sheet of moving material. We obtain the equations for the orthotropic Kelvin–Voigt material by inserting the viscoelastic stress-strain relations (27)–(29). Then, we insert the mixed Lagrange–Euler representations (25) for the material derivative in the viscous

terms, and restrict the inserted terms to the steady state  $(\partial/\partial t \to 0)$ . The result is

$$\int_{\Omega} \left[ -\left[ C_{11} \varepsilon_{xx} + K_{11} \left( U_{x} \frac{\partial \varepsilon_{xx}}{\partial x} + U_{y} \frac{\partial \varepsilon_{xx}}{\partial y} \right) + C_{12} \varepsilon_{yy} + K_{12} \left( U_{x} \frac{\partial \varepsilon_{yy}}{\partial x} + U_{y} \frac{\partial \varepsilon_{yy}}{\partial y} \right) \right] \right. \\
+ \rho U_{x}^{2} \frac{\partial u}{\partial x} + \rho U_{y} U_{x} \frac{\partial u}{\partial y} \left] \frac{\partial \phi}{\partial x} d\Omega \\
+ \int_{\Omega} \left[ -\left[ C_{66} \gamma_{xy} + K_{66} \left( U_{x} \frac{\partial \gamma_{xy}}{\partial x} + U_{y} \frac{\partial \gamma_{xy}}{\partial y} \right) \right] + \rho U_{x} U_{y} \frac{\partial u}{\partial x} + \rho U_{y}^{2} \frac{\partial u}{\partial y} \right] \frac{\partial \phi}{\partial y} d\Omega \\
+ \rho \int_{\Omega} \left\{ \left[ U_{x} \frac{\partial u}{\partial x} + U_{y} \frac{\partial u}{\partial y} \right] \left[ \frac{\partial U_{x}}{\partial x} + \frac{\partial U_{y}}{\partial y} \right] \right\} \phi d\Omega \\
+ \int_{\partial\Omega} n_{x} \left[ \left[ C_{11} \varepsilon_{xx} + K_{11} \left( U_{x} \frac{\partial \varepsilon_{xx}}{\partial x} + U_{y} \frac{\partial \varepsilon_{xx}}{\partial y} \right) + C_{12} \varepsilon_{yy} + K_{12} \left( U_{x} \frac{\partial \varepsilon_{yy}}{\partial x} + U_{y} \frac{\partial \varepsilon_{yy}}{\partial y} \right) \right] \\
- \rho U_{x}^{2} \frac{\partial u}{\partial x} - \rho U_{y} U_{x} \frac{\partial u}{\partial y} \phi d\Gamma \\
+ \int_{\partial\Omega} n_{y} \left[ \left[ C_{66} \gamma_{xy} + K_{66} \left( U_{x} \frac{\partial \gamma_{xy}}{\partial x} + U_{y} \frac{\partial \gamma_{xy}}{\partial y} \right) \right] - \rho U_{x} U_{y} \frac{\partial u}{\partial x} - \rho U_{y}^{2} \frac{\partial u}{\partial y} \right] \phi d\Gamma \\
+ \int_{\Omega} F_{x} \phi d\Omega = 0 \\
(76)$$

and 
$$\int_{\Omega} \left[ -\left[ C_{66} \gamma_{xy} + K_{66} (U_x \frac{\partial \gamma_{xy}}{\partial x} + U_y \frac{\partial \gamma_{xy}}{\partial y}) \right] + \rho U_x^2 \frac{\partial v}{\partial x} + \rho U_y U_x \frac{\partial v}{\partial y} \right] \frac{\partial \psi}{\partial x} d\Omega$$

$$+ \int_{\Omega} \left[ -\left[ C_{21} \varepsilon_{xx} + K_{21} (U_x \frac{\partial \varepsilon_{xx}}{\partial x} + U_y \frac{\partial \varepsilon_{xy}}{\partial y}) + C_{22} \varepsilon_{yy} + K_{22} (U_x \frac{\partial \varepsilon_{yy}}{\partial x} + U_y \frac{\partial \varepsilon_{yy}}{\partial y}) \right] \right.$$

$$\left. + \rho U_x U_y \frac{\partial v}{\partial x} + \rho U_y^2 \frac{\partial v}{\partial y} \right] \frac{\partial \psi}{\partial y} d\Omega$$

$$+ \rho \int_{\Omega} \left\{ \left[ U_x \frac{\partial v}{\partial x} + U_y \frac{\partial v}{\partial y} \right] \left[ \frac{\partial U_x}{\partial x} + \frac{\partial U_y}{\partial y} \right] \right\} \psi d\Omega$$

$$+ \int_{\partial \Omega} n_x \left[ \left[ C_{66} \gamma_{xy} + K_{66} (U_x \frac{\partial \gamma_{xy}}{\partial x} + U_y \frac{\partial \gamma_{xy}}{\partial y}) \right] - \rho U_x^2 \frac{\partial v}{\partial x} - \rho U_y U_x \frac{\partial v}{\partial y} \right] \psi d\Gamma$$

$$+ \int_{\partial \Omega} n_y \left[ \left[ C_{21} \varepsilon_{xx} + K_{21} (U_x \frac{\partial \varepsilon_{xx}}{\partial x} + U_y \frac{\partial \varepsilon_{xx}}{\partial y}) + C_{22} \varepsilon_{yy} + K_{22} (U_x \frac{\partial \varepsilon_{yy}}{\partial x} + U_y \frac{\partial \varepsilon_{yy}}{\partial y}) \right] - \rho U_x U_y \frac{\partial v}{\partial x} - \rho U_y^2 \frac{\partial v}{\partial y} \right] \psi d\Gamma$$

$$+ \int_{\Omega} F_y \psi d\Omega = 0.$$

$$(77)$$

Finally, by inserting into (76) and (77) the definitions of the Cauchy strains from (23), the weak form component equations for FEM implementation are obtained. The final result is

$$\int_{\Omega} \left[ -\left[ C_{11} \frac{\partial u}{\partial x} + K_{11} (U_{x} \frac{\partial^{2} u}{\partial x^{2}} + U_{y} \frac{\partial^{2} u}{\partial x \partial y}) + C_{12} \frac{\partial v}{\partial y} + K_{12} (U_{x} \frac{\partial^{2} v}{\partial x \partial y} + U_{y} \frac{\partial^{2} v}{\partial y^{2}}) \right] \\
+ \rho U_{x}^{2} \frac{\partial u}{\partial x} + \rho U_{y} U_{x} \frac{\partial u}{\partial y} \right] \frac{\partial \phi}{\partial x} d\Omega \\
+ \int_{\Omega} \left[ -\left[ C_{66} (\frac{\partial u}{\partial y} + \frac{\partial v}{\partial x}) + K_{66} (U_{x} (\frac{\partial^{2} u}{\partial x \partial y} + \frac{\partial^{2} v}{\partial x^{2}}) + U_{y} (\frac{\partial^{2} u}{\partial y^{2}} + \frac{\partial^{2} v}{\partial x \partial y})) \right] \\
+ \rho U_{x} U_{y} \frac{\partial u}{\partial x} + \rho U_{y}^{2} \frac{\partial u}{\partial y} \right] \frac{\partial \phi}{\partial y} d\Omega \\
+ \int_{\partial \Omega} n_{x} \left[ \left[ C_{11} \frac{\partial u}{\partial x} + K_{11} (U_{x} \frac{\partial^{2} u}{\partial x^{2}} + U_{y} \frac{\partial^{2} u}{\partial x \partial y}) + C_{12} \frac{\partial v}{\partial y} + K_{12} (U_{x} \frac{\partial^{2} v}{\partial x \partial y} + U_{y} \frac{\partial^{2} v}{\partial y^{2}}) \right] \\
- \rho U_{x}^{2} \frac{\partial u}{\partial x} - \rho U_{y} U_{x} \frac{\partial u}{\partial y} \right] \phi d\Gamma \\
+ \int_{\partial \Omega} n_{y} \left[ \left[ C_{66} (\frac{\partial u}{\partial y} + \frac{\partial v}{\partial x}) + K_{66} (U_{x} (\frac{\partial^{2} u}{\partial x \partial y} + \frac{\partial^{2} v}{\partial x^{2}}) + U_{y} (\frac{\partial^{2} u}{\partial y^{2}} + \frac{\partial^{2} v}{\partial x \partial y})) \right] \\
- \rho U_{x} U_{y} \frac{\partial u}{\partial x} - \rho U_{y}^{2} \frac{\partial u}{\partial y} \right] \phi d\Gamma + \int_{\Omega} F_{x} \phi d\Omega = 0$$
(78)

and

$$\int_{\Omega} \left[ -\left[ C_{66} \left( \frac{\partial u}{\partial y} + \frac{\partial v}{\partial x} \right) + K_{66} \left( U_{x} \left( \frac{\partial^{2} u}{\partial x \partial y} + \frac{\partial^{2} v}{\partial x^{2}} \right) + U_{y} \left( \frac{\partial^{2} u}{\partial y^{2}} + \frac{\partial^{2} v}{\partial x \partial y} \right) \right) \right] \\
+ \rho U_{x}^{2} \frac{\partial v}{\partial x} + \rho U_{y} U_{x} \frac{\partial v}{\partial y} \right] \frac{\partial \psi}{\partial x} d\Omega \\
+ \int_{\Omega} \left[ -\left[ C_{21} \frac{\partial u}{\partial x} + K_{21} \left( U_{x} \frac{\partial^{2} u}{\partial x^{2}} + U_{y} \frac{\partial^{2} u}{\partial x \partial y} \right) + C_{22} \frac{\partial v}{\partial y} + K_{22} \left( U_{x} \frac{\partial^{2} v}{\partial x \partial y} + U_{y} \frac{\partial^{2} v}{\partial y^{2}} \right) \right] \\
+ \rho U_{x} U_{y} \frac{\partial v}{\partial x} + \rho U_{y}^{2} \frac{\partial v}{\partial y} \right] \frac{\partial \psi}{\partial y} d\Omega \\
+ \int_{\partial \Omega} n_{x} \left[ \left[ C_{66} \left( \frac{\partial u}{\partial y} + \frac{\partial v}{\partial x} \right) + K_{66} \left( U_{x} \left( \frac{\partial^{2} u}{\partial x \partial y} + \frac{\partial^{2} v}{\partial x^{2}} \right) + U_{y} \left( \frac{\partial^{2} u}{\partial y^{2}} + \frac{\partial^{2} v}{\partial x \partial y} \right) \right) \right] \\
- \rho U_{x}^{2} \frac{\partial v}{\partial x} - \rho U_{y} U_{x} \frac{\partial v}{\partial y} \right] \psi d\Gamma \\
+ \int_{\partial \Omega} n_{y} \left[ \left[ C_{21} \frac{\partial u}{\partial x} + K_{21} \left( U_{x} \frac{\partial^{2} u}{\partial x^{2}} + U_{y} \frac{\partial^{2} u}{\partial x \partial y} \right) + C_{22} \frac{\partial v}{\partial y} + K_{22} \left( U_{x} \frac{\partial^{2} v}{\partial x \partial y} + U_{y} \frac{\partial^{2} v}{\partial y^{2}} \right) \right] \\
- \rho U_{x} U_{y} \frac{\partial v}{\partial x} - \rho U_{y}^{2} \frac{\partial v}{\partial y} \right] \psi d\Gamma + \int_{\Omega} F_{y} \psi d\Omega = 0 . \tag{79}$$

Equations (78)–(79) represent the weak form of the classical strong form equations (36)–(37), when consideration is restricted to the steady state.

We see that the weak form contains second derivatives in the viscous terms. Thus, to enforce integrability (see e.g. [29]), C1 continuity of basis functions across element boundaries is required in the viscoelastic case.

The corresponding pure elastic equations are obtained by setting  $K_{11}=K_{12}=K_{21}=K_{22}=K_{66}=0$ . Explicitly, we have

$$\int_{\Omega} \left[ -\left[ C_{11} \frac{\partial u}{\partial x} + C_{12} \frac{\partial v}{\partial y} \right] + \rho U_{x}^{2} \frac{\partial u}{\partial x} + \rho U_{y} U_{x} \frac{\partial u}{\partial y} \right] \frac{\partial \phi}{\partial x} d\Omega 
+ \int_{\Omega} \left[ -\left[ C_{66} \left( \frac{\partial u}{\partial y} + \frac{\partial v}{\partial x} \right) \right] + \rho U_{x} U_{y} \frac{\partial u}{\partial x} + \rho U_{y}^{2} \frac{\partial u}{\partial y} \right] \frac{\partial \phi}{\partial y} d\Omega 
+ \rho \int_{\Omega} \left\{ \left[ U_{x} \frac{\partial u}{\partial x} + U_{y} \frac{\partial u}{\partial y} \right] \left[ \frac{\partial U_{x}}{\partial x} + \frac{\partial U_{y}}{\partial y} \right] \right\} \phi d\Omega 
+ \int_{\partial\Omega} n_{x} \left[ \left[ C_{11} \frac{\partial u}{\partial x} + C_{12} \frac{\partial v}{\partial y} \right] - \rho U_{x}^{2} \frac{\partial u}{\partial x} - \rho U_{y} U_{x} \frac{\partial u}{\partial y} \right] \phi d\Gamma 
+ \int_{\partial\Omega} n_{y} \left[ \left[ C_{66} \left( \frac{\partial u}{\partial y} + \frac{\partial v}{\partial x} \right) \right] - \rho U_{x} U_{y} \frac{\partial u}{\partial x} - \rho U_{y}^{2} \frac{\partial u}{\partial y} \right] \phi d\Gamma 
+ \int_{\Omega} F_{x} \phi d\Omega = 0$$
(80)

and

$$\int_{\Omega} \left[ -\left[ C_{66} \left( \frac{\partial u}{\partial y} + \frac{\partial v}{\partial x} \right) \right] + \rho U_{x}^{2} \frac{\partial v}{\partial x} + \rho U_{y} U_{x} \frac{\partial v}{\partial y} \right] \frac{\partial \psi}{\partial x} d\Omega \right] 
+ \int_{\Omega} \left[ -\left[ C_{21} \frac{\partial u}{\partial x} + C_{22} \frac{\partial v}{\partial y} \right] + \rho U_{x} U_{y} \frac{\partial v}{\partial x} + \rho U_{y}^{2} \frac{\partial v}{\partial y} \right] \frac{\partial \psi}{\partial y} d\Omega \right] 
+ \rho \int_{\Omega} \left\{ \left[ U_{x} \frac{\partial v}{\partial x} + U_{y} \frac{\partial v}{\partial y} \right] \left[ \frac{\partial U_{x}}{\partial x} + \frac{\partial U_{y}}{\partial y} \right] \right\} \psi d\Omega \right] 
+ \int_{\partial\Omega} n_{x} \left[ \left[ C_{66} \left( \frac{\partial u}{\partial y} + \frac{\partial v}{\partial x} \right) \right] - \rho U_{x}^{2} \frac{\partial v}{\partial x} - \rho U_{y} U_{x} \frac{\partial v}{\partial y} \right] \psi d\Gamma \right] 
+ \int_{\partial\Omega} n_{y} \left[ \left[ C_{21} \frac{\partial u}{\partial x} + C_{22} \frac{\partial v}{\partial y} \right] - \rho U_{x} U_{y} \frac{\partial v}{\partial x} - \rho U_{y}^{2} \frac{\partial v}{\partial y} \right] \psi d\Gamma 
+ \int_{\Omega} F_{y} \psi d\Omega = 0 . \tag{81}$$

Equations (80) and (81) are the basis of the two-dimensional solutions presented in this study. Only first derivatives of the displacements are needed. Hence, C0 finite elements are sufficient.

Once a basis is chosen for u and v, it is straightforward to insert the Galerkin representation of both displacement variables into (80) and (81). Then, considering that the virtual displacement  $\phi$  is arbitrary, we use the basis functions of u as the test functions  $\phi$  and the basis functions of v as the test functions  $\psi$ , obtaining the discrete equation system for the classical Galerkin method. Essential and natural boundary conditions are then applied as usual.

If the velocity field is divergence-free, we may omit the third line in both equations, and the velocity can be allowed to have finite discontinuities across element boundaries. In all other cases, the velocity field must have C0 continuity across element boundaries, in order to enforce integrability of (80)–(81). This is because the terms on the third line of both equations follow directly from the weak form of the problem, and thus cannot be applied only in element interiors (as is done e.g. when second derivatives appear in certain numerical stabilization schemes for flow problems; see e.g. [17]).

#### 7. Coupling between drive velocity and in-plane displacement

If some edges of the domain are free, the velocity field driving the material may cause them to move. Each free edge will move until the velocity field becomes tangential to it. For small deformations, it is possible to avoid deforming the mesh, and still account for the slightly changed direction of the free edges by using approximate methods.

When free edges are present, the effective velocity field in the material is a priori unknown. In a sense, the driving velocity field modifies itself when the free edge deformations are taken into account. Thus, instead of taking U as a prescribed external velocity, this quantity may be redefined to fulfill two tasks. First, it will still be based on the driving velocity field, but secondly, the modified U will also take

into account the small deformations in the free edges, including the effect that the free edge deformations have on the effective velocity field inside the domain.

We will use a deformation-based approach. Consider the differential plane element in the small-displacement regime, see Figure 5. The original, undeformed edges are given by the vectors  $\mathbf{x} = (\mathrm{d}x,0)$  and  $\mathbf{y} = (0,\mathrm{d}y)$ . After deformation by a displacement field  $\mathbf{u} = (u,v)$ , these edges become (with the help of a first-order two-dimensional Taylor expansion of each of the fields u and v)

$$\tilde{\mathbf{x}} = \left( \left( 1 + \frac{\partial u}{\partial x} \right) dx, \ \frac{\partial v}{\partial x} dx \right) , \quad \tilde{\mathbf{y}} = \left( \frac{\partial u}{\partial y} dy, \ \left( 1 + \frac{\partial v}{\partial y} \right) dy \right) . \tag{82}$$

The projection

$$\mathbf{u}_{\mathbf{X}} = \mathbf{M} \cdot \mathbf{u}_{\mathbf{x}} \,, \tag{83}$$

that is,

$$\begin{bmatrix} u_X \\ u_Y \end{bmatrix} = \begin{bmatrix} \hat{\mathbf{x}} \cdot \hat{\mathbf{X}} & \hat{\mathbf{y}} \cdot \hat{\mathbf{X}} \\ \hat{\mathbf{x}} \cdot \hat{\mathbf{Y}} & \hat{\mathbf{y}} \cdot \hat{\mathbf{Y}} \end{bmatrix} \begin{bmatrix} u_x \\ u_y \end{bmatrix}, \tag{84}$$

transforms any vector field u given in  $\mathbf{x}=(x,y)$  components into  $\mathbf{X}=(X,Y)$  components. Here  $\hat{\mathbf{x}}$  and  $\hat{\mathbf{y}}$  (respectively  $\hat{\mathbf{X}}$  and  $\hat{\mathbf{Y}}$ ) are the unit vectors giving the directions of the axes of the (x,y) (respectively (X,Y)) coordinate systems. The boldface subscript denotes which coordinate system the quantity is given in, and the plain subscripts denote components.

We choose

$$\hat{\mathbf{X}} := (1,0), \quad \hat{\mathbf{Y}} := (0,1),$$
 (85)

and accounting for both direction and (small) length changes of the reference vectors, take  $\hat{\mathbf{x}}$  and  $\hat{\mathbf{y}}$  as

$$\hat{\mathbf{x}} := \tilde{\mathbf{x}}/\mathrm{d}x \;, \quad \hat{\mathbf{y}} := \tilde{\mathbf{y}}/\mathrm{d}y \;. \tag{86}$$

using the fact that the original lengths of the sides of the differential element were dx and dy.

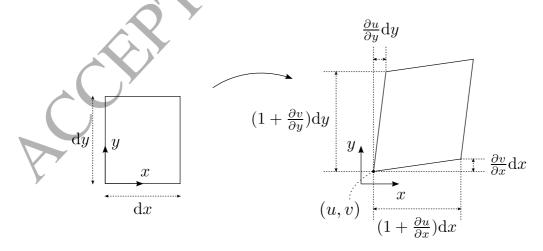


Figure 5: Small deformation of a differential plane element.

Now, instead of prescribing the velocity field  $U_X$ , we prescribe  $U_x$  (along the deformed coordinate directions), and then project it to the capital-X axes using (84). Then we set  $U = U_X$  in equations (80)–(81).

Using (84)–(86), we obtain the corrected velocity components as

$$U_X = \hat{x}_1 U_x + \hat{y}_1 U_y = \left(1 + \frac{\partial u}{\partial x}\right) U_x + \frac{\partial u}{\partial y} U_y ,$$

$$U_Y = \hat{x}_2 U_x + \hat{y}_2 U_y = \frac{\partial v}{\partial x} U_x + \left(1 + \frac{\partial v}{\partial y}\right) U_y .$$
(87)

Here  $\hat{x}_j$  denotes the jth component of the vector  $\hat{\mathbf{x}}$ , and we have used (85). In case of pure axial input flow,  $U_x = U_{\text{in}}$  and  $U_y = 0$ , we have

$$U_X = \left(1 + \frac{\partial u}{\partial x}\right) U_{\text{in}} ,$$

$$U_Y = \frac{\partial v}{\partial x} U_{\text{in}} .$$
(88)

In the present study, equations (88) have been used for computing the velocity field  $\mathbf{U}_{\mathbf{x}}$ .

Note that the correction (87) makes the effective velocity field  $U_{\mathbf{X}}$  space-dependent, even if the original input  $U_{\mathbf{x}}$  is not, due to the space dependence of  $\mathbf{u}$ . Hence, we cannot assume U to be constant; terms involving its derivatives must be retained in the equations.

The corrected velocity field will violate mass conservation, because the calculation is based on geometric considerations only. Thus, a further correction is required to preserve mass. We will need two different approaches depending on whether the material is compressible or not.

For a compressible material, we may use the mass conservation equation (1) to compute the velocity field inside the domain, using for boundary conditions the edge data for the normal component (in (X,Y) coordinates) of the proposed velocity given by (87).

To do this, one can view the mass conservation equation as diffusion of velocity potential. Assume that the velocity field is irrotational ( $\nabla \times \mathbf{v} = 0$ ); this is reasonable for steady state flow of a solid. In a steady state,  $\partial \rho / \partial t = 0$ . We have

$$\nabla \cdot (\rho \mathbf{v}) = 0. \tag{89}$$

After multiplication by a test function  $\chi$ , integrating over the domain  $\Omega$ , and applying the divergence theorem, we have the weak form

$$\int_{\partial\Omega} (\mathbf{n} \cdot \mathbf{v}) \,\rho \,\chi \,d\Gamma - \int_{\Omega} \rho \,\nabla \chi \cdot \mathbf{v} \,d\Omega = 0 \;. \tag{90}$$

Note that here  $\rho$  does not need to be constant. Next, let us use the fact that an

irrotational vector field has a scalar potential:

$$\mathbf{v} = \nabla V \,, \tag{91}$$

where V is a scalar-valued function. Inserting (91) into (90) obtains

$$\int_{\partial\Omega} (\mathbf{n} \cdot \nabla V) \,\rho \,\chi \,d\Gamma - \int_{\Omega} \rho \,\nabla \chi \cdot \nabla V \,d\Omega = 0 \,. \tag{92}$$

which is a steady-state diffusion equation (Poisson equation) for the velocity potential V. The natural boundary condition is

$$(\mathbf{n} \cdot \nabla V)\rho = q , \qquad (93)$$

i.e. one can prescribe the normal component of mass flow as a known function q. Hence, if the density at the edge is known, it is possible to prescribe the normal component of velocity there, via use of the natural boundary condition.

Recall (7), which gives density in terms of the axial strains, which in turn are obtained from the displacements via (23). Thus the density field can be computed once the displacements are known. Any C0 elements can be used for representing V.

By computation of the scalar potential V, the tangential velocity component at the edges will adjust itself such that mass balance is satisfied. Furthermore, the computed velocity field  $\nabla V$  inside the domain will also satisfy mass balance.

Because we have a Poisson problem with purely natural boundary conditions, the solution V is unique only up to an arbitrary additive constant. This can be remedied by standard techniques. One that is easy to implement is to add a small reaction term  $\int_{\Omega} \epsilon \, \chi \, V \, \, \mathrm{d}\Omega$  to the left-hand side of (92), where  $\epsilon$  is a small constant (e.g.  $\epsilon = 10^{-8}$ ).

In the case of an incompressible material, in the steady state the mass conservation equation (1) becomes simply  $\nabla \cdot \mathbf{v} = 0$ , i.e., the velocity field of an incompressible material must be divergence-free.

Therefore, in this case we simply compute the divergence-free projection of the proposed velocity field (87). The projection can be obtained using the standard trick based on the Helmholtz decomposition. For completeness, let us review this briefly. Let  $\mathbf{U}_e$  be the expected velocity field before the correction for mass conservation. Define a scalar potential p such that

$$\Delta p = \nabla \cdot \mathbf{U}_e \ . \tag{94}$$

Since the laplacian  $\Delta(...) \equiv \nabla \cdot \nabla(...)$ , equation (94) is equivalent with

$$\nabla \cdot \nabla p = \nabla \cdot \mathbf{U}_e \ . \tag{95}$$

By rearranging terms, we have

$$\nabla \cdot (\mathbf{U}_e - \nabla p) = 0 , \qquad (96)$$

i.e. the difference  $U_e - \nabla p$  is divergence-free. Define the final velocity field as

$$\mathbf{U} = \mathbf{U}_e - \nabla p \ . \tag{97}$$

Then U is the divergence-free projection of  $U_e$ . As the boundary conditions, one may require

$$\mathbf{n} \cdot \nabla p = 0 \tag{98}$$

on all boundaries. This ensures that the normal component of the correction  $\nabla p$  vanishes at the boundaries. The solution will then adjust the tangential component at the boundaries to enforce the divergence-free property there; in the interior of the domain, both components are allowed to change.

Again, we have a Poisson problem with purely natural boundary conditions, and thus the solution p is unique only up to an arbitrary additive constant. This can be worked around as above. Let  $\epsilon$  be a small constant, e.g.  $\epsilon = 10^{-8}$ . The final weak form of the projection problem is

$$-\int_{\Omega} \nabla p \cdot \nabla \chi \, d\Omega + \epsilon \int_{\Omega} p \, \chi \, d\Omega + \int_{\partial \Omega} (\mathbf{n} \cdot \nabla p) \chi \, d\Gamma = \int_{\Omega} (\nabla \cdot \mathbf{U}_e) \chi \, d\Omega \,, \tag{99}$$

where  $\chi$  is a test function. Any C0 elements can be used for representing p.

Strictly speaking, if u is represented by C0 elements, the divergence of  $U_e$  (which depends on the second derivatives of u) is not integrable due to singularities at the element boundaries. However, the right-hand side of (95) is basically arbitrary, as is also the velocity field U when viewed as input to (80)–(81). The only requirement is that either  $\nabla \cdot \mathbf{U} \equiv 0$ , or alternatively, that  $\nabla \cdot \mathbf{U}$  is integrable. Thus, we may choose to omit the singularities when defining the auxiliary quantity p by (95); any practical solver code will then see  $\mathbf{U} = \mathbf{U}_e - \nabla p$  as a divergence-free velocity field.

This also has implications for the compressible case treated further above. There, nonzero divergence was allowed for the velocity field (via local variations in density). Now, however, the divergence of the velocity field is present in the original equations (80)–(81) themselves, and we cannot ignore singularities if we are to represent the problem correctly. If the presented free-edge approximation is used, then u and v must be represented using elements having C1 continuity across the element edges. In the present study, we have chosen to ignore this issue by using a divergence-free velocity field, allowing us to omit the problematic terms in (80)–(81).

There is one final issue that must be accounted for. The presented free-edge approximation will make (80)–(81) nonlinear, since the effective velocity field U depends on the displacement u. Thus an iterative process must be introduced to find the solution.

#### 8. Numerical results

In this section, we will present numerical results from both the one- and two dimensional models. In the one-dimensional case, the full viscoelastic model is used. The two-dimensional study is focused on the inertial contribution in the pure elastic case.

In the one-dimensional model, the strain and stress states can be calculated analytically using equations (51) and (55). Using the parameter values  $E_x=2.5\cdot 10^7~\mathrm{N/m^2}$ ,  $\eta_x=4.0\cdot 10^5~\mathrm{Ns/m^2}$ ,  $U_0=10~\mathrm{m/s}$ , span length  $\ell=1.0~\mathrm{m}$  and endpoint strain  $\varepsilon_T=0.03$ , the results shown in Figures 6–8 are obtained.

Figure 6 shows the strain distribution along the length of the span based on both analytical and numerical solutions, which are seen to agree. The analytical solution is given by equation (51), and the numerical solution is reproduced from the previous study [36]. As was pointed out in the analysis, the strain distribution along the travelling viscoelastic span differs from the traditional constant strain that is observed for purely elastic materials [69].

Figure 7 displays the viscous and elastic stress contributions, and the resulting total stress. Even though the strain distribution is not constant, the stress distribution is a combination of elastic and viscous contributions as per equation (52), and it is approximately constant (in Figure 7, to within 0.4%). The stress increases very slightly towards the traction line A2; this is a consequence of the acceleration of the web due to higher outlet velocity,  $U_2 > U_1$  [36].

Figure 8 shows the total stress at the point  $x=\ell$ , where the stress reaches its maximum, for different span lengths  $\ell$  and drive velocities  $U_0$ . The effect of the span length on the web stress state is visible. Basically, the shorter the processing time of the viscoelastic span is (for a given endpoint strain  $\varepsilon_T$ ), the higher is the response of time-dependent viscous force component. This is seen from equation (54) by considering the point  $x=\ell$  and letting  $\ell$  tend to zero. Then, while the elastic stress stays constant at the value  $E_x\varepsilon_T$ , the viscous stress (after cancelling by  $e^{-k\ell}$ ) increases without bound.

In case of the two-dimensional model, a purely elastic travelling two-dimensional sheet was investigated numerically using finite elements. For the setup, see Figure 9. The unknowns were the displacements u and v. The element type used was Q2 (quadratic and quadrangular). Isoparametric mapping (i.e. also Q2) was used for the coordinates. The auxiliary potential p for divergence-free velocity projection was represented using Q1 (bilinear quadrangular) elements. The classical Galerkin method was used: for each unknown quantity, its own basis functions were used as the test functions.

The mesh was set up as a uniform cartesian grid with  $16 \times 16$  elements for each of the unknowns. Discretizations were produced automatically from (80)–(81) and (99). Dirichlet boundary conditions, prescribing displacements, were enforced by the elimination technique. Homogeneous natural (i.e. zero Neumann) boundary conditions required no action on part of the implementation. The discretization lead to 32895 global degrees of freedom in the linear problem (u and v only), and 37120 in the nonlinear problem (u, v and auxiliary potential p).

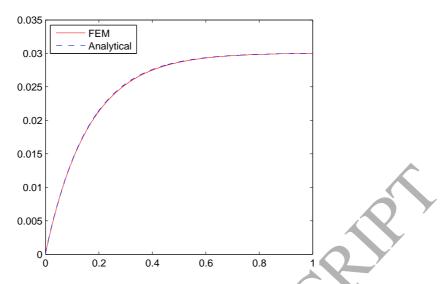


Figure 6: Numerical and analytical solutions of strain distribution along the length of the span. The analytical solution is given by equation (51). [36]

The solution of the corresponding linear problem for the stationary elastic sheet (U=0) was used as the initial guess for u, and fixed-point iteration was used to refine the solution of the nonlinear problem of the moving sheet. At each fixed-point iteration, the linear equation system was solved by a direct solver. The velocity field U was computed starting from a purely axial inflow, as described above. See equations (88), (97) and (99).

A common desktop computer with 4 CPU cores was used for the finite element computations. With MPI-parallelized matrix assembly, each nonlinear problem took about one minute of wall time. The implementation was based on the scientific Python software stack.

Problem parameter values used in the study are listed in Table 1. The shear modulus based on the geometric average,

$$G = \frac{\sqrt{E_1 E_2}}{2(1 + \sqrt{\nu_{12}\nu_{21}})} , \qquad (100)$$

was used for both the isotropic and orthotropic materials. Equation (100) is sometimes known as the Huber value for the shear modulus, after M. T. Huber who proposed this relation for orthotropic materials [26, 27, 28].

The boundary conditions for the displacements u and v were

$$u(y) = v(y) = 0$$
 at  $x = 0$ ,  $0 < y < b$ , (101)

$$u(y) = u_0$$
 at  $x = \ell$ ,  $0 < y < b$ , (102)

and zero Neumann for v at  $x = \ell$ , 0 < y < b. In (102),  $u_0$  is a prescribed constant value for the displacement. Note that at the outflow edge, only u is fixed; v is determined by the zero Neumann condition.

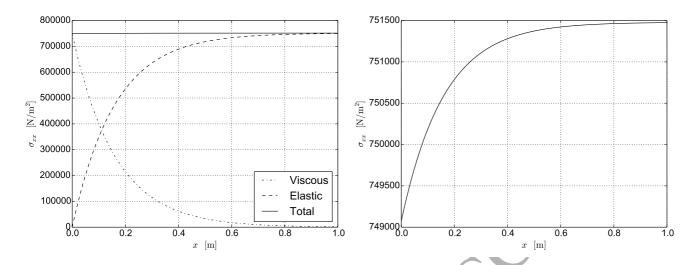


Figure 7: Analytical solution of the stress distribution along the length of the span, using equations (51), (54) and (55). *Left*: Elastic and viscous stress contributions, and the total stress. *Right*: Zoomed-in view of the total stress. Note the vertical scales.

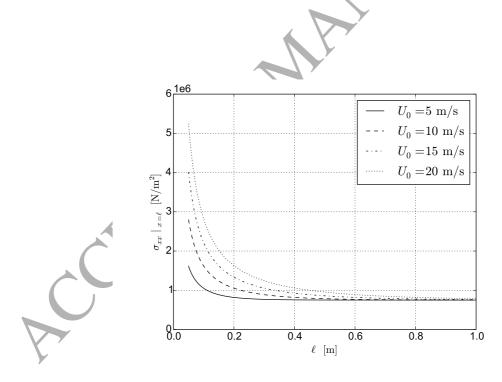


Figure 8: Effect of span length  $\ell$  and drive velocity  $U_0$  on the total stress  $\sigma_{xx}$  at the point  $x = \ell$ , where the stress reaches its maximum value. [36]

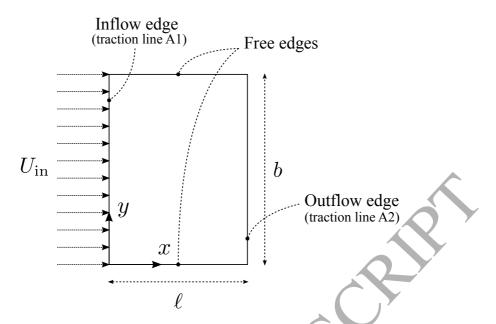


Figure 9: Setup for the two-dimensional numerical investigation. Elastic material enters from the left at velocity  $U_{\rm in}$ , and flows through the domain at a velocity determined by the interaction of  $U_{\rm in}$  and the approximated behaviour of the free edges.

On the free edges,  $0 < x < \ell$ , y = 0 and  $0 < x < \ell$ , y = b, the zero Neumann condition was used for both u and v. The zero Neumann condition (weakly) prescribes zero normal component for the effective stress tensor (73), i.e.

$$\sigma_{\text{eff}} \cdot \mathbf{n} \equiv \left[ \boldsymbol{\sigma} - \rho (\mathbf{U} \otimes \mathbf{U} \cdot \nabla \mathbf{u})^{\text{T}} \right] \cdot \mathbf{n} = 0 .$$
 (103)

Explicitly, accounting for our geometry and spelling out the component form, from (81) we can read the following condition corresponding to v at the outflow boundary:

$$C_{66}\left(\frac{\partial u}{\partial y} + \frac{\partial v}{\partial x}\right) - \rho U_x^2 \frac{\partial v}{\partial x} - \rho U_y U_x \frac{\partial v}{\partial y} = 0 \quad \text{at} \quad x = \ell, \quad 0 < y < b , \qquad (104)$$

and from (80) and (81), the following conditions corresponding to u and v (respectively) on the free boundaries:

$$C_{66}\left(\frac{\partial u}{\partial y} + \frac{\partial v}{\partial x}\right) - \rho U_x U_y \frac{\partial u}{\partial x} - \rho U_y^2 \frac{\partial u}{\partial y} = 0 \quad \text{at} \quad 0 < x < \ell, \quad y = \{0, b\} , \quad (105)$$

$$C_{21}\frac{\partial u}{\partial x} + C_{22}\frac{\partial v}{\partial y} - \rho U_x U_y \frac{\partial v}{\partial x} - \rho U_y^2 \frac{\partial v}{\partial y} = 0 \quad \text{at} \quad 0 < x < \ell, \quad y = \{0, b\} \ . \tag{106}$$

The boundary conditions for the auxiliary potential p were (98) on all boundaries. As was mentioned, this requires that the normal component of the velocity correction vanishes at the boundaries.

The problem was solved numerically for all 24 input value combinations result-

Table 1: Parameter values used in the two-dimensional numerical study. All 24 combinations resulting from the choices of  $U_{\rm in}$  (4 values), b (3 values) and the two materials were investigated. The shear modulus based on the geometric average (equation (100)) was used for both the isotropic and orthotropic materials. Note elastic compatibility  $E_1\nu_{21}=E_2\nu_{12}$  (see e.g. [38]).

$U_{\rm in} \left[ {\rm m/s} \right]$	$\ell \ [m]$	b [m]	$ ho  [\mathrm{kg/m^3}]$	$u_0$ [m]
0	0.5	0.1	800	$0.03 \cdot \ell$
15		0.5		
25		2.5		
50				

	$E_1 \ [10^7  \mathrm{Pa}]$	$E_2 \ [10^7  \mathrm{Pa}]$	$\nu_{12}$	$\nu_{21}$
Isotropic	2.5	2.5	0.3	0.3
Orthotropic	5.0	1.25	0.6	0.15

ing from the choices of  $U_{\rm in}$  (4 values), b (3 values) and the two materials, as listed in Table 1.

Figure 10 shows the y-directional displacement profile (displacement v) along the free edge  $0 < x < \ell$ , y = b for representative isotropic and orthotropic cases for both stationary and moving materials.

Figures 11–14 display contour plots of the displacements u and v for a material moving at  $U_{\rm in}=50$  m/s, which is a representative example of the nonlinear case. Also shown is the contribution of the nonlinear part, i.e. the differences  $u-u_{\rm lin}$  and  $v-v_{\rm lin}$ . Here  $u_{\rm lin}$  and  $v-v_{\rm lin}$  and

For all investigated cases, it was observed that for the displacements, strains, and stresses, the difference between the nonlinear and linear solutions is small, when compared to the maximum value of the linear solution. While varying the magnitude of  $U_{\rm in}$  changed the magnitude of this difference, the qualitative shape of the nonlinear contribution remained the same.

All effects were observed to become more pronounced for small aspect ratios  $\ell/b$ , i.e. short and wide spans. The span length  $\ell$  was kept constant, and the width b was varied; hence in the figures, large values of b correspond to small aspect ratios.

In all investigated cases, inertial effects caused additional elastic contraction of the travelling span, on top of the contraction already observed for a stationary material. This effect can be seen especially clearly in the *y*-directional displacement profile, Figure 10.

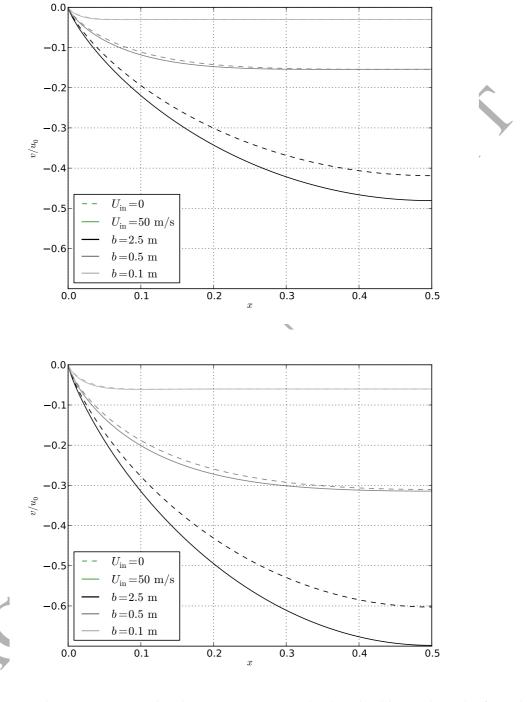


Figure 10: Displacement v, normalized to  $u_0$  (see equation (102) and Table 1), along the free edge  $0 < x < \ell$ , y = b. Top: isotropic. Bottom: orthotropic. Line style indicates inflow velocity  $U_{\rm in}$ , darkness indicates span width b.

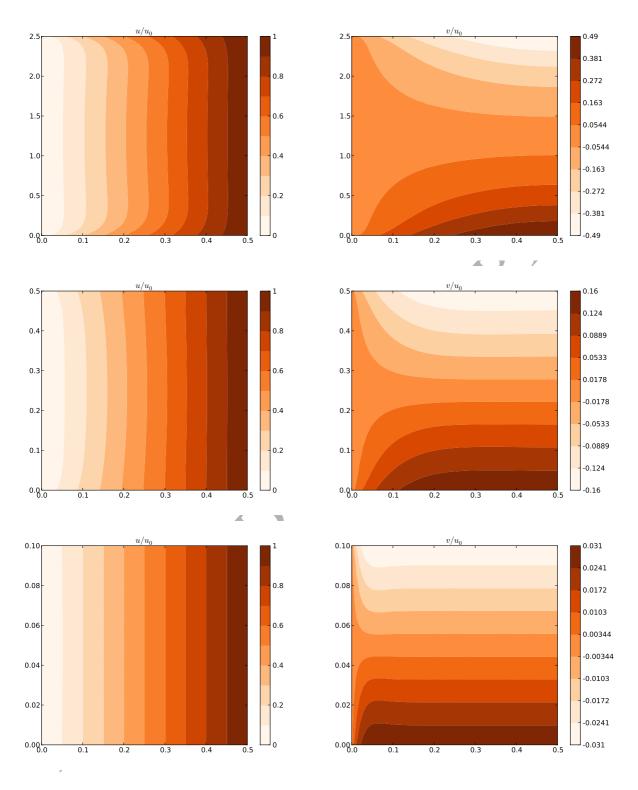


Figure 11: Displacements u and v, normalized to  $u_0$  (see equation (102) and Table 1). Isotropic,  $U_{\rm in}=50$  m/s (representative). Top to bottom:  $b=2.5,\ 0.5,\ 0.1$  m ( $\ell/b=1/5,\ 1,\ 5$ ).

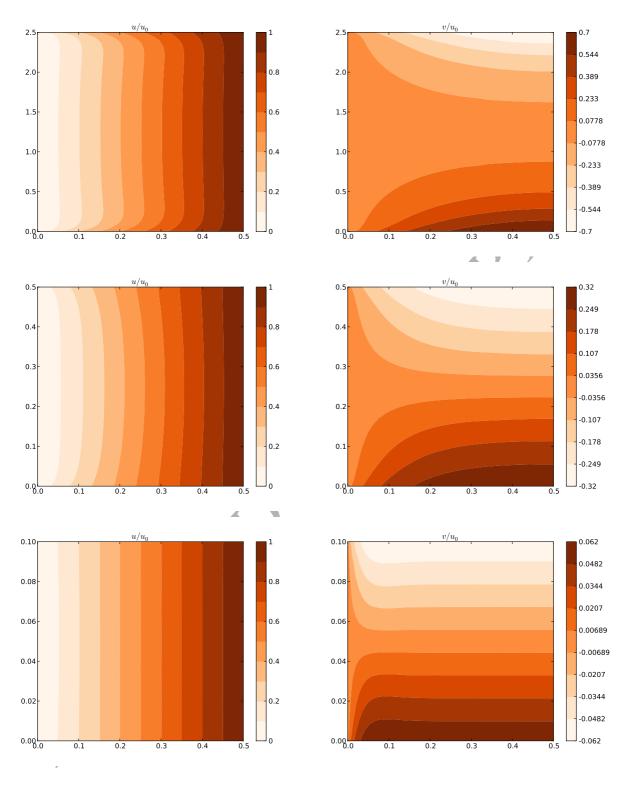


Figure 12: Displacements u and v, normalized to  $u_0$  (see equation (102) and Table 1). Orthotropic,  $U_{\rm in}=50$  m/s (representative). Top to bottom:  $b=2.5,\ 0.5,\ 0.1$  m ( $\ell/b=1/5,\ 1,\ 5$ ).

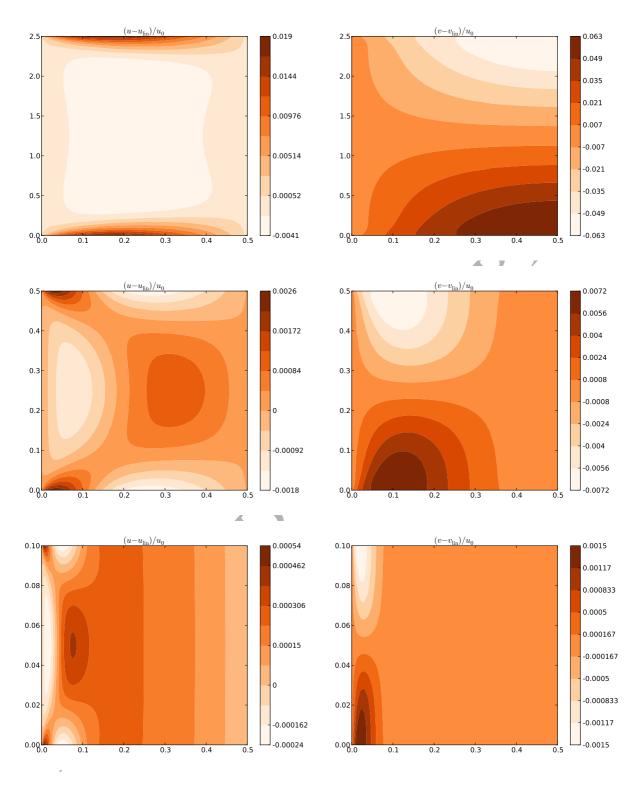


Figure 13: Nonlinear contribution to displacements, normalized to  $u_0$  (see equation (102) and Table 1). Isotropic,  $U_{\rm in}=50$  m/s (representative). *Top to bottom:*  $b=2.5,\ 0.5,\ 0.1$  m ( $\ell/b=1/5,\ 1,\ 5$ ).

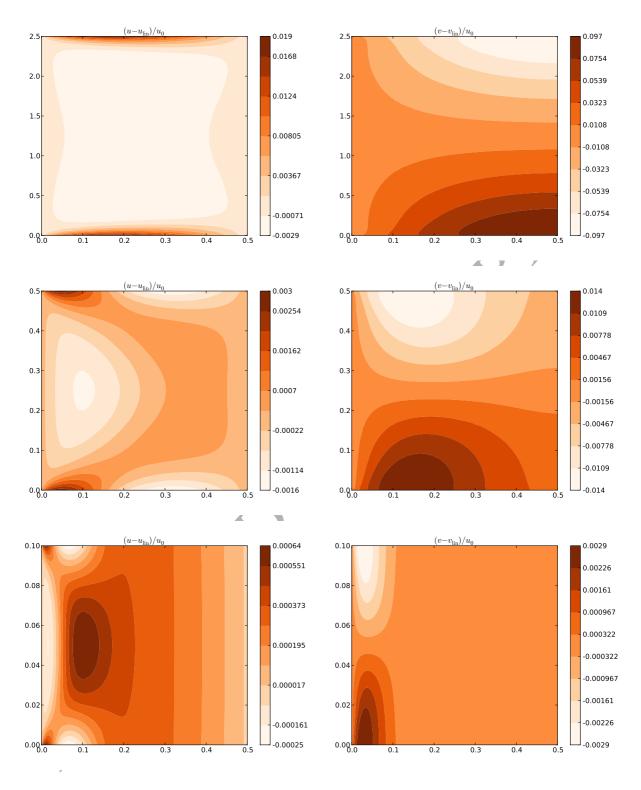


Figure 14: Nonlinear contribution to displacements, normalized to  $u_0$  (see equation (102) and Table 1). Orthotropic,  $U_{\rm in}=50$  m/s (representative). *Top to bottom:*  $b=2.5,\ 0.5,\ 0.1$  m ( $\ell/b=1/5,\ 1,\ 5$ ).

#### 9. Conclusion

In the present study, one-dimensional and two-dimensional models for the steady state in-plane flow of axially moving thin solid sheets were considered.

By a mass conservation argument, an expression was derived for the strain induced by a velocity difference between subsequent supporting rollers. This is relevant especially in paper production, where this velocity difference is the mechanism used for generating the axial tension in the travelling paper web.

The one-dimensional analysis was performed using the Kelvin–Voigt viscoelastic solid material model, while the two-dimensional analysis concentrated on a linear elastic material.

The one-dimensional model was solved analytically, leading to expressions for the strain and stress distributions. It was found that when viscous effects and axial motion are both present, the strain and stress follow a saturating exponential curve across the free span. This is unlike in the classical linear elastic case, where the strain and stress take on a constant value across the whole span, regardless of whether or not the material is subjected to axial motion. Furthermore, for short free spans, the total stress at the span endpoint, in the viscoelastic model, increases significantly (compared to its elastic value) due to viscous effects.

In the two-dimensional model, the focus of the study was on inertial effects. A key finding is that inertial effects cause additional contraction on top of the Poisson contraction already observed for a stationary material.

Due to Poisson contraction of the travelling material subjected to axial strain, the driving velocity field is not exactly axial, but instead it follows the shape of the free edges. This was modeled as a small velocity perturbation on top of the axial driving velocity field. A method was proposed to approximately find the final velocity field when an initial velocity field (without contraction-dependent corrections) is given.

The two-dimensional model presented is always nonlinear. The reason for this is that due to mass conservation and the behaviour of free edges, the total material velocity U is dependent on the displacement u, which further is dependent on the same in-plane velocity.

The present study includes some limitations. The model was studied in a steady state, which does not allow for temporal variations in the flowing medium preceding the observation span under study. To include such effects, a time-dependent formulation must be used. Also, a purely elastic material law was used for determining the strain caused by a velocity difference between subsequent rollers; to properly account for viscoelastic materials in a time-varying model, this must be generalized to the viscoelastic case.

The velocity field used in the model is currently assumed to affect the in-plane flow of the material "as such", without the loss of stability or deflections in the out-of-plane direction (which are considered in e.g. [3, 4, 9, 54]).

From a physical viewpoint, the use of Dirichlet boundary conditions in two-dimensional web handling models is problematic. In a two-dimensional model, there is no dimension in the thickness direction. Thus, the Dirichlet boundary conditions affect the whole thickness of the web, even though in reality all web handling

systems are based on surface traction. In the interior of the web, the actual axial displacement (e.g. at the mid-surface) will differ from the value set by the Dirichlet boundary condition, which in physical terms represents a prescribed displacement at the surface.

A logical future step to continue along the present line of study would be to consider the effects viscoelasticity also in the two-dimensional case, since the web velocity and viscous properties are closely connected [36].

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